

THE RICE INSTITUTE

THE FAILURE OF CONCRETE
UNDER TRIAXIAL STRESSES

by

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INTRODUCTION

This study was done to investigate the behavior of concrete under three-dimensional stresses with two objects in mind. The first object was to determine the effect of the lateral stresses on the longitudinal strength of a concrete specimen with empty voids. The other was to determine the effect of the interstitial pressure on the strength of the material.

To achieve these two objects cylindrical concrete specimens were subjected to triaxial compressive stresses with two lateral stresses constant and equal and the specimens brought to failure under axial compression. Some specimens were tested with empty voids and others with a liquid under pressure in the pores. To create the first of these conditions a rubber tubing was used on the specimen to protect it from the liquid pressure. In the second case the liquid was free to penetrate the specimen voids. The relative difference, between the two, in the strength of the material gives the effect of liquid under pressure in the pores.

The influence of lateral stresses on the axial strength of a concrete member has a direct application in reinforced concrete columns. Also, this influence would help us to understand the behavior of concrete under various stresses and be useful in formulating a theory of failure. The case of liquid under pressure in the voids has a direct application to structures that are subject to hydraulic pressure. This is called the effect of pore-water pressure, or uplift, and it is important to understand its effect on stability for the design of any hydraulic structure. It is suggested by the latest theories that this effect can be

determined only by experimental means. There is at present no definite idea about either the exact potential area of surface or the part of it on which the pore pressure acts.

PREVIOUS INVESTIGATIONS

A material may fail in one of two ways: One way is a brittle failure in which first there is an elastic behavior when stresses are proportional to deformations. This phase is followed by fracture. The second way is called ductile and has another range between the elastic range and fracture. In this range small changes of stress are usually accompanied by large deformations.

Several theories are used to explain the failure of materials. These may be briefly summarized as follows:

Maximum-stress theory: Failure occurs when maximum normal stress on any plane exceeds a limiting value for that material.

Maximum-strain theory: Failure occurs when the strain at any point exceeds the failing strain of the material in a simple tensile test.

Maximum-distortion-energy theory: There is a limit to the amount of distortion energy that a body can absorb and remain elastic.

Besides these there is another group of failure theories which are called planes-of-least-resistance theories. According to these failure occurs along a plane where resistance to shearing stress is smallest. In this group is Coulomb's Internal Friction Theory, in which the limiting value of shearing stress, s , consists of a constant shearing strength, c , and a resistance of the nature of friction, f , which is proportional to the normal stress, p , acting on the plane. Thus

$$s = c + y(f)$$

Also in this group is the maximum-shear theory. According to this theory failure is produced when the maximum shear stress reaches a limiting

value. This theory was proposed by Guest.

According to Mohr's theory normal stress plays a role in failure as well as shear and their interdependence is defined by:

$$s = f(y)$$

This equation is determined experimentally. The value of the intermediate principal stress plays no role in this theory. A more specific study of it will be given in the analysis section.

One of the earliest works in the field is by F. D. Adams⁽¹⁾ who in 1901 published results of triaxial tests on rock. In his case it was impossible to measure the confining pressure because of friction as he used a steel jacket to confine the specimen.

The next work was done by Karman and Böker⁽²⁾ on marble, under confining pressures, mainly to test the correctness of Mohr's theory. In these tests compressive stresses were applied in three different ways. In the first case two equal principal stresses were higher than the third principal stress, and in the second case two equal principal stresses were lower than the third principal stress. In the third case, test specimens were also subjected to tension. The first two cases indicated that the intermediate principal stress had some effect on the failure. Higher shearing stresses were present at failure in the first case than in the second. In the third case tensile stresses were produced and whenever they exceeded a limiting value failure occurred, which contradicts Mohr's theory.

Probably the most comprehensive study was done at the University of Illinois and was published in Bulletin 185.⁽²⁾ In these tests three different series of tests were done. In the first group concrete specimens were subjected to two-dimensional compression. In the second and

third groups specimens were subjected to triaxial compression. In the second group the two lateral stresses were lower than the axial stress; in the third group they were higher. Using these tests Brandtzaeg⁽³⁾ developed a failure theory, which is explained in the analysis and also in Appendix 2.

In the biaxial tests at the University of Illinois, specimens were compressed laterally with oil pressure with no stress in the third direction. To prevent failure due to axial tensile stress specimens were covered with plaster of Paris, then by a thin sheet of annealed brass and protected from oil pressure that way. It was concluded that the strength of concrete under biaxial compression is at least as great as under uniaxial compression and in some cases 25 to 50% greater. Failure occurred suddenly as a total fracture across a plane normal to the axis of the cylindrical specimen. It was also concluded that Saint Venant's maximum-strain theory is inapplicable to concrete for the following reason: if we compute the strains due to combined compression from the equations of elasticity and stipulate that the tensile strain should not exceed a fixed limit, we find a strength in biaxial compression one-half that of uniaxial compression, which is not the case.

Series 3a presented in Bulletin 185 is much the same as the author's jacketed tests and will be referred to later. In Series 3B two lateral stresses were higher than the axial. It was concluded from Series 3a and 3b that the compressive strength of concrete increases with confining pressure, and that the amount of the increase is 4.1 times the smallest principal stress.

In 1936 Griggs⁽⁴⁾ made important contributions to the design of high pressure equipment. His equipment is basically the same as that designed

by Jones⁽⁵⁾ and used by the author in this study.

At this point it is best to introduce some contributions on the effect of pore pressure, which, in regard to hydraulic structures, is often mentioned as uplift. Earlier theories considered the water in the pores of an hydraulic structure due to either imaginary or visible cracks. Modern views disregard these abstract views and accept the fact that water filters through the pores of masonry or concrete. It can be demonstrated that water can be made to pass through any kind of concrete by passing through the pores of the material, whether cracks are present or not.

Mr. Fillunger⁽⁶⁾ was the first person to give a scientific approach to the modern views which later were revised by Professor Terzaghi. Professor Fillunger's approach is usually presented as three laws. He broke some specimens in free air and some subjected to confining pressure, so that water under pressure was present in the material's pores. He did not get any appreciable difference in ultimate load between the two cases, so he states that as long as the water in the pores remains at constant pressure it does not affect the resistance of the material. For his second law the author refers to Terzaghi's⁽⁶⁾ example, which is a pier under water. See figure 1.

Let J = unit weight of the material of the pier

W = weight of the shaded portion

Then the stress at section AB will be equal to

$$\frac{W}{A} = HJ$$

Water in the pores will relieve some of this pressure. The relief in stress at section AB will be equal to the intensity of the water pressure at that level, h_p , times the porosity of the material, which we

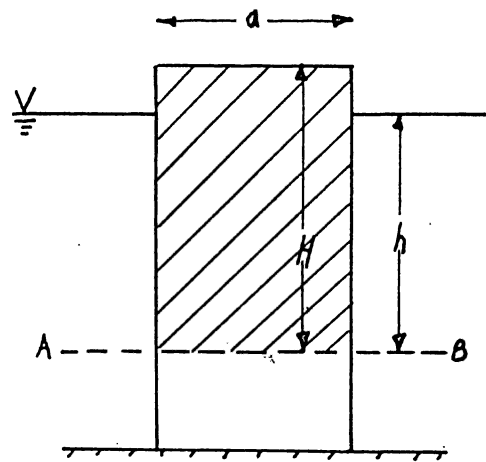


Figure 1

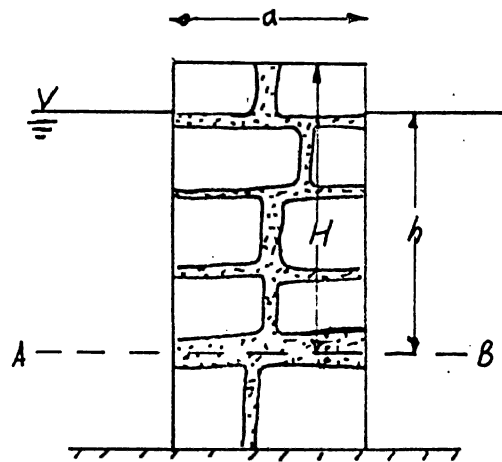


Figure 2

shall call n . Thus the relief in stress at section AB is:

$$h \rho n$$

where ρ = unit weight of water.

But we see also that the water in the pores of the material will cause an increase in stress at section AB which would be equal to ρ times the volume of water in pores above AB per unit area of section, viz., $h n$. This increase in stress is thus

$$h \rho n$$

The total stress at section AB is therefore

$$HJ = h \rho n + h \rho n = HJ$$

This analysis shows that there is no difference between the cases of a pier under water and a pier completely out of water, as when there is no water the stress at section AB would still be HJ . At this point Prof. Fillunger introduced his third law by differentiating between the average porosity of the material, n , and the porosity of mortar, n_1 . He reasoned that when failure occurs, the surface of rupture goes through the binding material such as mortar which binds the aggregates.

We refer to the same example of a pier under water, figure 2, which this time is formed of blocks of stone bound by more porous mortar.

Using the same coefficients otherwise, pressure in the pores of section

AB is: $h \rho n_1$

Weight of water absorbed at height h is:

So the final stress will be;

$$HJ - h \rho (n_1 - n)$$

There is now an uplift force which is equal to:

$$P = h \rho (n_1 - n) \quad (1)$$

and we define an uplift factor, U_f , as follows:

$$u_f = n_1 - n \quad (2)$$

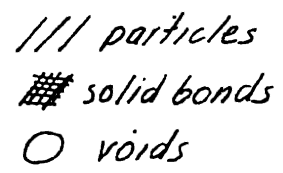
Fillunger's conclusion, expressed as Equation 2, has undergone some basic changes to which we shall refer.

Prof. K. Terzaghi^(7, 8) in 1945 presented an analysis investigating the mechanical causes of failure of concrete with and without pore pressure. His analysis depends on the fact that splitting governs the failure and is due to an intergranular wedge action. He explains this action as follows:

Figure 3a is a section of a porous quasi-isotropic material which is acted on by a vertical force Y . This force will destroy the solid bonds of rounded particles by producing tension at point a. Professor Terzaghi concludes that if such a material fails by splitting, the failure in the bonds subject to tension will precede the failure in the others. Also, the average normal stress on that section is zero, but as the material is only statistically isotropic the normal stress is only statically equal to zero. If all the negative forces are divided by one-half of the section at a vertical section, negative scatter stress y_s , is obtained. There is a compressive force per unit area on the other half. If there is an average stress, y , on the section, then one-half of the section is acted on by $y + y_s$, and the other by $y - y_s$. As soon as $-y_s$ becomes equal to the tensile strength, y_t of the material, failure is produced by the scatter stress, y_s . Prof. Terzaghi calls it splitting stress and expresses failure with equation 3 as:

$$y_s = y + y_t \quad (3)$$

In triaxial compression, lateral pressure Y_2 , is the normal stress, y , so that $y = Y_2$ in equation 3. Then failure in this case occurs when the splitting stress equals the normal stress (confining pressure in this



scattering

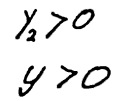


Figure 3e

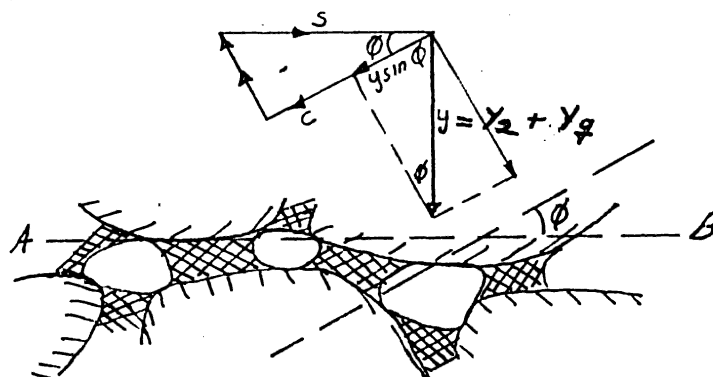


Figure 4

case) plus the tensile strength, y_t of the material. Also, since the total axial load, Y_1 , produces y_s , splitting stress increases in proportion to total axial load. At $Y_2 = 0$ the failing value of Y_1 is equal to the compressive strength, y_c , of the material and:

$$y_s = y_t \frac{Y_1}{y_c}$$

Combining with equation 3, also remembering that, $y = Y_2$

$$Y_1 = y_c + Y_2 \frac{y_c}{y_t} \quad (4)$$

Equation 4 represents the amount of total axial load we should expect in a triaxial loading.

According to this theory, failure is always intergranular, even in triaxial loading. In Figure 4, plane AB is the plane of failure of the material under triaxial loading, but at a particular point on this plane the details of the intergranular connections will cause the direction of movement associated with failure to deviate from plane AB by an angle ϕ . The total unit shearing resistance to failure, S_1 , is the sum of two independent resistances, S and S_f , where

S = shearing stress required to sever the intergranular connections

S_f = frictional resistance of grains per unit area

Thus

$$S_1 = S + S_f \quad (5)$$

From a consideration of the vector diagrams in Figure 4 we see that

$$S = \frac{c}{\cos \phi} + (Y_2 + Y_q) \tan \phi$$

and

$$S_f = (Y_2 + Y_q) \frac{f}{\cos \phi}$$

where: c = unit cohesive strength (assumed independent of stress)

f = coefficient of friction of grain on grain

y = normal stress on AB, which is actually equal to the confining pressure, Y_2 , plus the excess unidimensional pressure, Y_q .

Further in his theory Professor Terzaghi considers the effect of pore water pressure and concludes that the liquid under pressure in the pores equal to lateral pressure will decrease the effect of the confining pressure as the pressure transmitted from solid to solid will no longer be Y_2 , but instead will be $Y_2(1 - n_a)$, where n_a is the boundary porosity. This decrease depends on the degree of continuity of intergranular bond. It is the ratio of that area of the potential surface of failure which is in contact with the liquid in the pores and the total area of the surface. This factor should be introduced in Equations 4 and 5 when there is pore water pressure.

Professor Terzaghi's most important contribution in the uplift field is his disregarding of the plane section analysis of Professor Fillunger to find the uplift factor, as failure might be in a zig-zag way. Experimental methods are necessary to determine the uplift factor as it is not necessarily related to physical constants. As a result, equation 2 for uplift factor becomes

$$u_f = n_a - n \quad (6)$$

Professor Terzaghi comes out with an n_a of unity in his experiments.

Mr. S. Leliavsky⁽⁹⁾ also has made important contributions in the field of uplift. His specimens had a hole in the middle just like a

small pipe with extremely thick walls. Mr. Leliavsky wanted to be sure that the pressure was the same along the entire face of rupture. First he subjected the test piece to a force, F , then he raised the outer pressure (the pressure outside the specimen) to a constant. Then he waited until the inner pressure, which he measured with a manometer, was brought to the same level. He repeated this procedure of raising the pressure by increments until failure occurred. At the moment of failure the uplift must overcome both the force, F , and the resistance, z , of the material. The total pressure in the pores is pA times a reduction factor, n_a , which is called the boundary porosity by Terzaghi. Thus the final equation will be:

$$n_a p A = z + F \quad (7)$$

Mr. Leliavsky tested several specimens at various forces, from which he determined the value of n_a to be 0.91.

Mr. Leliavsky introduced another concept in uplift theory so that Equation 6 changes into:

$$u_f = n_a - n_v \quad (8)$$

The factor n_v replaces the volumetric porosity, n , of concrete. n_v is a constant which takes into account the fact that there is always a certain amount of moisture present in concrete which reduces the amount of moisture concrete can absorb. Mr. Leliavsky finds that the water that may be added over normal moisture content is 7% of the volume of the materials in his tests. He got that result by exposing his specimens to the sun. Thus he found n_v to be 0.07 and u_f to be 0.84.

Mr. McHenry⁽¹⁰⁾ has done tests on 6-inch by 12-inch specimens with both jacketed and unjacketed specimens. His testing machine was designed for a pressure of up to 125,000 psi, but the lateral pressure used in

his tests were held to a maximum of 1,800 psi. He used kerosene or nitrogen gas to confine his specimens. He used the Mohr-Coulomb theory for his analysis and he reduced the normal stresses in that equation, for the case ofunjacketed specimens, by the amount of the pore pressure times the boundary porosity. His equation becomes:

$$s = c + (y - Y_2 n_a) \tan \theta$$

where θ = the angle of internal friction.

He gets an n_a value of 1.02. The effect of lateral pressure Y_2 on axial load Y_1 he found to be:

$$Y_1 = y_c + 5.848 Y_2 \quad \text{without pore pressure}$$

$$Y_1 = y_c + 0.772 Y_2 \quad \text{with pore pressure}$$

Besides the men mentioned, there are others who have made contributions in these fields. The ideas presented here cover what the author considers to be the best approaches to the problems in question.

SPECIMEN PREPARATION

Four different types of mixture were tested in this study. Test specimens for each mixture were drilled from the same concrete block by means of a diamond drill and then were cut to a standard length with a diamond saw. Specimens obtained this way had a diameter varying between 0.505 and 0.503 inches and a length varying between 1.007 and 1.0085 inches. These variations are between mixtures regardless of mixture. As far as the individual specimens were concerned these dimensions were consistent. Before cutting specimens to the required length, they were placed into an aluminum tubing with an inside diameter of 0.509 inches and a length of 2 inches. A slotted hole was cut through one side of the tube for easy entrance and for better contact during the cutting process as it could be better pressed in the vise of the saw that way. Specimens obtained that way had a very smooth surface and sharp edges.

To control the moisture content, specimens were dried at 105°C. for a period of three days, after which they did not lose any more weight. Right after drying, the specimens were vacuumed for the same length of time. Checks of the weight showed further decrease in weight during that process. After this routine, no more weight checks were made. For each mixture, moisture control of the specimens to be tested jacketed and unjacketed was done separately. Specimens tested with empty voids were placed into a plastic tubing after vacuumizing. Specimens to be tested without a jacket were saturated with oil while under vacuum. Oil used for this purpose was the one used in the test chamber, namely No. 1 motor oil.

Drilling was the most time consuming part of the specimen

preparation. In order to get exact outside diameters and smooth surfaces various combinations of pressure and feeding had to be tried. The author was unsuccessful in his efforts to get mortars containing only cement and water, as all the specimens broke either during drilling or cutting. Also quite a number of specimens broke when they were removed from the aluminum tubes.

EQUIPMENT USED AND TEST PROCEDURE

Equipment used in this study is that designed by Jones⁽⁵⁾. It is based on the apparatus of Griggs⁽⁴⁾. Figures 32 and 33 show the pressure testing equipment used in this study. It is designed for a maximum of 20,000 psi oil pressure. Oil pressure is applied with an hydraulic jack, J, which is placed firmly on plate, L₃. Piston, P₂, has access to the test chamber through a cavity. Specimen, S, is placed into test chamber of the pressure vessel, V. Specimen stays on a stopper, R, which has a hole through it so that oil has an access from the cavity where the piston, P₂, moves, to the cavity where the test specimen stays. Upper piston P₁ enters the cavity through the threaded plug, T. The specimen stays between the upper piston and the stopper. It is placed into a brass sleeve which has a conical portion that fits to the conical surfaces of the stopper and piston. The rest of the brass sleeve is cylindrical and fits to a soft steel slug, used as a transition medium between specimen and piston or specimen and stopper, and extends 1/8 inch onto the specimen. This last 1/8 inch is slotted.

The pressure testing equipment is placed between the platens of the testing machine, which applied the axial load. Axial load is transferred to the specimen by means of upper piston, P₁, whose spherical head fits to a bore at the center of plate, L₁. Plates L₁ and L₃ are yoked together by three equally spaced columns, Y, and as the specimen deflects they are free to move. Three columns, C, connect the pressure vessel to plate L₂. One bar equipped with a slotted hole is fitted to each one of the pistons. The other edges contact the plunger of a dial indicator, D₁ for upper piston and D₂ for lower piston, which are mounted to the sides of the pressure vessel.

An 1/8 inch "O" ring is placed in the groove in the bore of the plug T. A 3/16 inch "O" ring is placed in a groove in the pressure vessel where it contacts the plug T. Lower piston has a groove for a 1/8 inch "O" ring.

A specimen is placed into the test cavity with sleeves and steel slugs as explained. A specimen to be tested jacketed is fitted into plastic tubing which covers the specimen, sleeves and part of the upper piston (Figure 34a). After placing the specimen into the test chamber, oil is added to the level of the "O" ring and then nut T is threaded into place. There are small grooves in the nut threads through which air in the test cavity escapes. When the test equipment is all assembled there is a small hydrostatic pressure. Then the pressure equipment is placed in the testing machine and an initial end load of a little more than 68 pounds is added. Jones found out that the part of the equipment which the upper piston supports is 32 pounds, but there are also the friction forces due to "O" rings of 100 pounds. Then the oil pressure is applied. As oil has access to the pressure chamber through the stopper R, a condition of equal hydrostatic pressure is created all over the specimen. The head of the testing machine is lowered to keep the excess pressure. The procedure is repeated until the desired oil pressure is reached and from then on the axial load is applied with increments of 200 pounds. At higher axial loads the increment is increased. After a few readings it was possible to record readings without stopping the testing machine.

At each increment dial indicator readings are recorded. As the two pistons are yoked together, the lower piston P_2 withdraws as much

as upper piston P_1 advances into the chamber. This helps to keep the hydrostatic pressure constant in the test vessel. If the volume of the specimen changes, remembering that the lower piston could move independently from the upper piston, though it repeats all the movements of the upper piston, (See Figure 32), we get a difference between piston movements. Multiplying this difference by the piston area will give the volume change in the chamber.

For the upper piston a 0.0001-inch dial indicator with a travel of 0.3 inches was used, and for the lower piston a 0.001-inch dial with a travel of one inch was used.

In order to get the exact volume changes and deflections we must correct the readings obtained during the experiment as those readings include deflection of upper piston steel slugs, pressure vessel and so on. For this purpose a steel specimen, with known modulus of elasticity and the same shape and dimensions as the concrete specimens, was tested under exactly the same conditions as concrete specimens. Axial load was kept below the yield stress of the steel specimen. From this data, we will get the apparatus deflection which is the correction to be applied to the piston readings. In these calibration tests the difference of piston readings did not show any significant amount, which points out a zero volume change and also could be taken as a proof about the correctness of future volume change readings of concrete specimens.

The correction to be applied is 1×10^{-4} inches for every 100 pounds of excess load up to 3000 pounds, 8×10^{-5} inches from 3000 to 5000 pounds, 6×10^{-5} from 5000 up to 7500 pounds, and from then on 2×10^{-5} pounds for every 100 pounds excess load. Decrease in those values is due to decrease in apparatus deflections. Also corrections are rounded

up to 10^{-4} of an inch. There is no definite difference for different oil pressures so that the above corrections are used for all oil pressures. Only at zero oil pressure the deflection from 0 to 100 pounds excess load is 7×10^{-4} and from then on goes the same as the others. So these are the values applied for correction.

RESULTS OF TESTS

In this study four different concrete mixtures were tested in three dimensional loading. For each mixture both jacketed and un-jacketed specimens were tested at eight different oil pressures: 0, 2,500; 5,000; 7,500; 10,000; 12,500; 15,000 and 17,500 pounds per square inch. A series of 2 by 4 inch cylinders in compression and standard tensile briquets were tested for each mixture. Concrete blocks, 2 by 4 inch cylinders and tensile briquets for each mixture were kept in a moist closet for 28 days. All mixtures were hand mixed.

Each mixture is separated into two groups which are differentiated with alphabetic letters C, D, E, F, G, H, I, J. C is for the first mixture unjacketed and D for its jacketed specimens; E is for the second mixture unjacketed and so on. Table 1 shows characteristics of the 2 by 4 inch cylinders.

TABLE 1

Mixture	*Group	w/c	**Compressive strength (psi)	Tensile strength(psi)
0.75: 0.92: 0.25	C & D	0.98	4,210	420
1:2 Mortar	E & F	0.60	4,960	430
1:3 Mortar	G & H	0.88	2,910	260
1: 1: 0.64	I & J	1.00	2,320	185

* C, E, G, I, groups tested unjacketed

D, F, H, J groups tested jacketed

** Compressive strengths of $\frac{1}{2}$ by 1 inch cylinders is higher than these.

See tables 10 and 11.

For all the mixtures the same kind of cement and sand were used, which are: ASTM Type 1 Portland Cement with a specific gravity of 2.63; sand which has a sieve analysis of 100% passing 40 mesh sieve and 100% retained on 60 mesh sieve.

For groups C and D (mixture 0.75: 0.92: 0.25) a pozmix cement blend of 75-25 was used. Pozallan used in this blend is called pozmix A (fly ash), which is a reactive silicate that reduces the free lime in portland cement and renders it more resistant to sea water's corrosion. Pozmix A has a specific gravity of 2.46. For groups I and J (mixtures 1: 1: 0.64) an expanded clay aggregate is used. Expanded clay has a specific gravity of 2.63.

Most of the data and computations are presented in Appendix II. Columns 1, 2, 3, and 6 show the applied oil pressure, applied excess load, and dial indicator readings for the lower and upper pistons respectively. These are the readings taken during the experiment. The other columns of figures are computed quantities. Column 4 is the difference between two successive lines of Column 3 with a correction applied. (The amount and need for correction is explained in the section "Equipment Used and Test Procedure".). Column 5 gives corrected lower piston deflection readings found for each load by adding Column 3 readings up to that point. Column 7 is the amount of correction needed at each load increment in the upper piston readings. Column 8 gives corrected upper piston readings (Column 6 - Column 7) or Unit Deformation of the Specimen in inches per inch. Column 9 (Column 8 - Column 5) times the piston area (both pistons have the same area, viz., 0.37 square inch) gives the volume change of the specimen, Column 10.

* Forunjacketed specimens and zero oil pressure we cannot get the volume changes with this equipment. At zero oil pressure specimens tested are without oil, as the only way to check that the chamber is full of oil is to have some hydrostatic pressure in it due to excess oil. We cannot do this for simple compression tests, for if we do, it will affect the results. And, as long as the chamber is not full, we cannot get volume changes. For the unjacketed specimens as the oil is free to move into or out of the pores we cannot measure volume changes in the specimen with this apparatus. Attention is called to the data in Appendix II where some of the volume changes for unjacketed specimens are computed. These data show a variation around a straight vertical line which proves the above hypothesis. Jacketed specimens did not have any contact with the oil so that we get results directly.

Tables 2 to 5 are for jacketed specimens and 6 to 9 are for unjacketed specimens. All the specimens tested are presented with their confining pressure and their excess load over confining pressure. Also, when possible, permanent unit deformations are given in axial and lateral directions, which are a decrease and increase in dimension respectively. This is done only for jacketed specimens which deflect beyond the scope of dial indicators and they always stay solid so that it is possible to get permanent deformations. Lateral deformations are measured at the approximate center of the specimen.

Considering the graphs, Figures 9-16, for jacketed specimens, we see that there is a plastic yielding even at the lowest lateral pressure. In unjacketed specimens all the failures are brittle. For jacketed specimens all groups deflected in a similar way except group J which

* Refer to the section "Equipment Used and Test Procedure" to see the way volume changes are computed.

showed larger deflections. Deviations from average strength are mostly independent of oil pressure.

Volume changes for jacketed specimens are also plotted (Figs. 21-28) and show a decrease in volume.

Initial slopes of load-deformation curves are presented in Tables 2 through 9. Those lines are drawn as tangents to the straight bottom part of the load-deformation curves (Figures 9-20). These are the slopes after excess load over confining pressure is applied and often do not include the very first one which is usually way off with respect to the others.

Tables 10 and 11 are the summaries of Tables 2 to 5 and 6 to 9 respectively. They contain the average of the initial slopes and maximum axial loads at constant oil pressure. Average compressive strengths at zero oil pressure of all mixtures came out more than their corresponding 2 by 4 inch cylinder strengths, Table 1. The differences for Groups D, F, H and J are 13.5%, 12%, 8% and 6% respectively. Strengths of unjacketed specimens are more or less similar to corresponding jacketed ones, with no fixed pattern.

Average maximum unit loads in Tables 10 and 11 are plotted in Figure 8 against confining oil pressure, for both jacketed and unjacketed specimens. In group F (jacketed) the ultimate strength of the specimen at lower oil pressures increases 4.8 times the applied oil pressure; at higher oil pressures the increase in ultimate strength is 3.8 times the applied oil pressure. For group D the same increase factor for low and high oil pressures is 3.8 and 3.3 respectively; for group H it is 4.5 and 3.5; for group J it is 4.6 and 3.5. That is to say, for jacketed specimens the increase in axial load with increasing oil pressure is

not a straight-line relationship. A smooth curve through the data would show a greater slope at low pressures than at high pressures. For jacketed specimens the average increase of strength is about 3.9 times the oil pressure applied and straight lines representing that relation are drawn in Figure 8. Forunjacketed specimens an average increase in strength of 1.2 times the oil pressure is accepted, and straight lines representing this relation are drawn. For them, the increase in axial load with increasing oil pressure first follows a curve of increasing strength and after low pressures a straight line.

Actual angles of failure are measured for unjacketed specimens and presented in Appendix II with the compiled data. All jacketed specimens showed a ductile behavior. If the experiment was continued long enough, the jacketed specimens broke into pieces. The surface of failure usually made an approximate angle of about 30° with the longitudinal axis of the specimen. Angles that lines on the barrel-shaped specimens made with the longitudinal axis kept increasing as long as the experiment was carried on. Whenever an experiment was carried until the specimen shortened more than approximately 20% of its length, the brass sleeves were destroyed, so usually the author stopped the experiment before then.

Also sometimes it was not possible to measure the angles of unjacketed specimens due to breaking into pieces from the brass sleeves or during removal from the test chamber. For such cases the author only wrote that the piece was damaged.

In general the failures for group E happened as two pieces with an angle of failure of around 33° with the longitudinal axis. For this group usually one of the two pieces was larger than the other.

For group C failure consisted of two pieces with some longitudinal splitting and possibly with an average failure plane of 30° . For group G failure planes were around 28° with two major pieces and a few small pieces. For group I failure planes were around 23° with two major and several small pieces. For these last three groups the failure plane usually started from one corner and continued approximately diagonally across to the opposite corner.

There was usually a slight increase of strength, but not always, with increasing lateral pressures. There was considerable variation in failure angle within each group.

Group	Specimen Number	Oil Pressure 16. per sq. in.	Differential Load 16	Initial Slope by load Deformation Curve 1000 lb. per sq. in.	Permanent Deformation in.	Axial Lateral in.
F	1		1000	4100		
	2	0	1150	3400		
	3		1000	2900		
	4		1000	4100		
	5		1300	3700		
F	1		2880	1250	.094	.040
	2	2500	2980	1300	.082	.054
	3		3080	1460	.243	.150
F	1		4300	1460	.064	.037
	2		4280	930		
	3	5000	4400	780	.135	.060
	4		4500	1700		
F	1		+5200	780		
	2	7500	5420	1250	.114	.051
	3		5700	1280	.285	.159
F	1		6400	1700	.127	.067
	2	10000	6240	850	.218	.073
	3		7000	1130	.191	.087
F	1	12500	7800	1020	.199	.051
	2		8220	1340	.151	.074
	3		+8600	1460		
	4		+7380	1020		
F	1		9280	1460		
	2	15000	9640	1250	.154	.080
	3		+9300	925		
	4		9000	1300	.092	.041

Specimen No.	Oil Pressure (psi)	Differential Load (lb)	Initial Slope of Load Deformation Curve (lb./sq. in.)	Axial Deformation (in.)	Lateral Deformation (in.)
1	17500	12000	1700	.126	.047
2		9400	1600	.107	.072
3		9900	1020	.122	.058
4		11740	700		

* During test oil penetrated rubber tubing + specimen broken

Table 3
All Specimens tested Jacketed

Group	Specimen Number	Oil Pressure 16.per sq.in.	Differential Load 16	Initial Slope of Load Deformation Curve 1000 lb. per sq. in.	Permanent Deformation	
					Axial in.	Lateral in.
D	1		800	3400		
	2		1120	1140		
	3	0	1000	5300		
	4		1100	3960		
	5		780	2900		
D	1		2170	1140	.073	.026
	2	2500	3020	1020		
	3		+1840	1160		
D	1		*1800	1000	.179	.053
	2	5000	4220	1620	.151	.074
	3		*2000	1500		
D	1		+4200	1620	.181	.051
	2	7500	4400	900	.159	.083
	3		4500	1280		
D	1		6600	1460	.304	.130
	2	10000	*2800	1460		
	3		6400	1400	.164	.071

Cont'd. on Next Page.

Table 3 - Cont'd.
All Specimens Tested Tacketed

D	1	6600	1460	.304	.130
	2	*2800	1460		
	3	.6400	1400	.164	.071
D	1	7650	1130	.091	.032
	2	8400	1400	.474	.263
	3	+7620	700		
D	1	9300	1130	.240	.014
	2	+9000	1020		
	3	*2420	1050		
D	1	*9560	1020	.256	.090
	2	*3600	830		
	3	-9500	1320	.191	.062
	4	+9900	1020		

* During test oil penetrated rubber tubing
+ Specimen broken

All specimens tested Jacketed

Group	Specimen Number	Oil Pressure 16 per sq in	Differential Load 16	Initial Slope of Load Deformation Curve 1000 lb. per sq. in.	Permanent Deformation Axial in. Lateral in.
H	1		800	4100	
	2		520	1460	
	3	0	600	1610	
	4		650	3400	
	5		430	1200	
H	1		2820	1140	.064 .036
	2	2500	*1740	1460	
	3		2400	1400	.120 .041
H	1		4000	625	.213 .092
	2	5000	3800	930	
	3		3900	1020	
H	1		5280	900	.181 .053
	2	7500	5040	850	.12 .086
	3		+5200	850	
H	1		6620	900	
	2	10000	*3800	900	.246 .081
	3		5980	950	
	4		6300	950	
H	1		7830	930	.252 .081
	2	12500	7220	785	.243 .092
	3		+7580	1280	
H	1		7970	850	.032 .047
	2	15000	8600	1020	.153 .055
	3		9040	730	
H	1		9600	930	.171 .036
	2	17500	+9650	1020	
	3		+8600	1020	

* During test oil penetrated rubber tubing
+ Specimen broken

All Specimens Tested Jacketed

Group	Specimen Number	Oil Pressure 16 per sq in	Differential Load 16	Initial Slope of Load Deformation Curve 1000 lb per sq in	Permanent Deformation	
					Axial in	Lateral in
J	1		400	-		
	2		500	730		
	3	0	620	1460		
	4		430	850		
J				1200		
	1		2000	640	.181	.087
	2	2500	2460	780	.106	.057
	3		+2200	780		
J	1		3200	400	.238	.104
	2	5000	3460	680	.171	.061
	3		+2000	781	.644	.378
				850		
J	1		4820	550	.259	.081
	2	7500	4640	550	.257	.090
J	1		6400	300	.476	.200
	2	10000	6200	600	.249	.078
	3		6000	400	.193	.054
J	1		7400	400	.260	.079
	2	12500	7600	500	.253	.090
	3		*2420	500		
J	1		8600	500	.258	.059
	2	15000	8300	790	.263	.084
	3		8450	500	.304	.093
	4		8400	850		
J	1		8600	400	.219	.047
	2	17500	9200	520	.253	.078
	3		9200	405	.223	.041

* During test oil penetrated rubber tubing
+ Specimen broken

Table 6
All Specimens Tested Unpacketed

Group	Specimen Number	Oil Pressure 16 per sq.in.	Differential Load 16	Initial Slope of Load Deformation Curve 1000 lb per sq. in
C	1		1240	4500
	2	0	800	1500
	3		860	3050
C	1		1450	1020
	2	2500	1020	1700
	3		1410	2400
C	1		1760	1020
	2	5000	1340	1220
	3		1200	1530
C	1		1600	1330
	2	7500	1680	1530
	3		1470	1200
C	1		1520	1140
	2	10000	1050	1180
	3		1420	1910
C	1		1220	1140
	2	12500	1400	1180
	3		1600	990
C	1		1350	1180
	2	15000	1800	1330
	3		1600	1400
C	1		1300	1460
	2	17500	1800	1400
	3		1910	1400

Table η
All Specimens Tested Unjacketed

Group	Specimen Number	Oil Pressure	Differential Load	Initial Slope of Load Deformation Curve
		16 per sq.in.	16	1000 lb persq. in.
E	1		950	2300
	2	0	1300	4100
	3		1160	4500
E	1		1040	1460
	2	2500	1600	1460
	3		1600	1390
E	1		1800	1460
	2	5000	1400	1390
	3		1220	1100
E	1		1600	1850
	2	7500	1400	710
	3		1460	1850
E	1		1580	1840
	2	10000	1400	1450
E	1		1610	1860
	2	12500	1600	1860
	3		1800	1100
E	1		1500	1300
	2	15000	2000	1600
	3		1600	1600
	4		1700	1250
E	1		1600	1390
	2	17500	1840	1800
	3		1810	1920
			1700	1800

All specimens tested unjacketed

Group	Specimen Number	Oil Pressure 16 per sq. in.	Differential Load 16	Initial Slope of load Deformation Curve 1000 lb. per sq. in.
G	1		920	3050
	2	0	400	2550
	3		570	2300
G	1		920	1020
	2	2500	1120	1140
	3		1000	980
G	1		900	980
	2	5000	1330	1400
	3		1100	1000
G	1		1200	1930
	2	7500	1040	930
G	1		1150	1020
	2	10000	1400	1460
	3		1300	1000
G	1		1090	1000
	2	12500	1300	1400
	3		1600	1180
G	1		1100	780
	2	15000	1700	1280
	3		1350	1000
G	1		1450	1280
	2	17500	1160	1280
	3		1300	830

Table 9
All Specimens Tested Unjacketed

Group	Specimen Number	Oil Pressure 16 per sq. in.	Differential Load 16	Initial Slope of Load Deformation Curve 1000 lb. per sq. in.
I	1		400	1140
	2	0	500	1280
	3		300	1000
I	1		1240	850
	2	2500	620	930
	3		1140	785
I	1		1200	875
	2	5000	900	875
	3		920	825
I	1		1440	970
	2	7500	1260	875
	3		1220	1020
I	1		1050	570
	2	10000	1250	930
	3		1270	550
I	1		1220	640
	2	12500	1360	1180
	3		1280	550
I	1		1280	730
	2	15000	1400	1280
	3			
I	1		1300	680
	2	17500	1500	1330
	3		1480	1400

Group	Lateral Oil Pressure	Number of Specimens Tested	Differential Load	Maximum Axial Load	Average Initial Slope of Load Deformation Curves
	16 per sq. in.		16 per sq. in.	16 per sq. in.	1000 lb per sq. in.
E	0	3	5740	5740	3300
	2500	3	7190	9690	1440
	5000	3	7500	12500	1320
	7500	3	7850	15350	1140
	10000	2	7600	17600	1650
	12500	3	8500	21000	1600
	15000	3	8600	23600	1440
	17500	3	8900	26400	1480
C	0	3	4800	4800	3000
	2500	3	6590	9090	1700
	5000	3	7300	12300	1250
	7500	3	7200	14700	1680
	10000	3	6800	16800	1410
	12500	3	7200	19700	1110
	15000	3	8000	23000	1310
	17500	3	8500	26000	1440
G	0	3	3200	3200	2550
	2500	3	5200	7700	1050
	5000	3	5890	10890	1120
	7500	2	5710	13210	1430
	10000	3	6390	16390	1150
	12500	3	6800	19300	1170
	15000	3	6390	21390	1020
	17500	3	6700	24200	1130
I	0	3	2350	2350	1140
	2500	3	5100	7600	770
	5000	3	5250	10250	870
	7500	3	5620	13120	660
	10000	3	6100	16100	685
	12500	3	6530	19030	790
	15000	2	6850	21850	1000
	17500	3	7250	24750	1140

Table 11
Summary of all Groups Tested Jacketed

Group	Lateral Oil Pressure	Number of Specimens Tested	Differential Load	Maximum Axial Load	Average Initial Slope of Load Deformation Curves
	16 per sq. in.		16 per sq. in.	16 per sq. in.	1000 16 per sq. in.
F	0	5	5620	5620	4000
	2500	3	15200	17700	1340
	5000	4	22200	27200	1210
	7500	3	27600	35100	1100
	10000	3	34500	44500	1230
	12500	4	41000	53500	1210
	15000	4	46100	61100	1240
	17500	4	51800	69300	1260
D	0	5	4870	4870	3240
	2500	3	11800	14300	1110
	5000	3	21500	26500	1370
	7500	3	27500	35000	1260
	10000	3	33000	43000	1440
	12500	3	40000	52500	1070
	15000	3	46000	61600	1060
	17500	4	49000	66500	1040
H	0	5	3160	3160	2400
	2500	3	11800	14300	1330
	5000	3	19800	24800	860
	7500	3	26100	33600	870
	10000	4	32000	42000	700
	12500	3	38400	50900	1000
	15000	3	43500	58500	865
	17500	3	41000	64500	980
J	0	4	2480	2480	1100
	2500	3	11300	13800	735
	5000	3	16900	21900	680
	7500	2	24100	31600	550
	10000	3	31600	41600	435
	12500	3	38100	50600	465
	15000	4	43000	58000	655
	17500	3	45900	63400	449

COMPARISON AND ANALYSIS OF RESULTS

Maximum stress and strain theories fail to explain the behavior of materials under hydrostatic pressure. In the first case the high normal stresses which are present without failure as a result of lateral pressures and in the latter case the occurrence of large strains without failure contradicts these theories.

If the maximum shear theory were correct the enveloping curve on the Mohr circles of failure should be a horizontal straight line and the fact that this is not so could be seen from Figures 29-31. We can get such a case if the compressive and tensile stress at failure are equal.

The maximum-distortion-energy theory is more applicable to elastic failure as the ability of materials to withstand hydrostatic stresses without yielding contradicts it.

Coulomb's internal friction theory is the most applicable one so far. As could be seen from Figures 29-31, the relationship between maximum load and lateral load is more or less a linear one. Coulomb's theory has all the disadvantages of Mohr's theory, which will be discussed.

In this study Mohr's theory is used for analysis. Naturally the causes of failure are more complex than assumed by this theory but nevertheless it gives the relationships of stresses at failure quite accurately in a simple and pictorial way. According to it normal stress plays a role in failure as well as the shearing stress. This interdependence could be expressed by this equation

$$s = f(y)$$

This relationship is determined experimentally by drawing stress circles for different failing stresses and drawing a common envelope to them called the envelope of rupture, Figures 29-31. From these curves the angle of the plane of sliding could be determined so that ϕ is the angle between the longitudinal axis and the direction of weakness of elements which first fail by sliding. All points of every stress circle should be below that envelope of rupture. For every circle there is a point on the circle which gives the stresses on the plane of failure. Referring below we find those stresses to be

$$y = \frac{1}{2} \sin^2 \phi + \frac{1}{2} \cos^2 \phi$$

$$s = \frac{y_1 - y_2}{2} \sin 2\phi$$

These could be directly measured from the Mohr diagram too.

As for the case of the specimens tested with pore pressure, there is no point, as McCutchen⁽¹¹⁾ suggested, in drawing Mohr circles with pore pressure as a parameter. Instead effective pressure should be used. That is, only 31% of the confining pressure should be included in the computations instead of 100%. Due to pore pressure the strength that the bonds get from the lateral pressure is reduced, which directly depends on the degree to which pore pressure is effective. This point and the way 0.31 is found will be explained later in the analysis. The Mohr diagrams in Figures 31 are drawn according to that principle and they definitely have the interdependence equation as mentioned. This point corresponds to University of Illinois experiments too.

According to Mohr's theory materials are homogeneous and isotropic so that there is nothing in the material that would cause failure to happen in any particular direction. He assumes that there is a relation between the normal stress and the shearing stress in any plane which

governs the resistance to failure along that plane.

Another assumption of this theory completely disregards the effect of intermediate stress on the failure of materials. It reaches this conclusion since for any stress there are many planes that might have that stress (a vertical line at that stress on the Mohr circle represents all of those points) but only one plane at the same time has maximum shearing stress and failure will take place on that plane if it ever does with that stress. The intersection of the enveloping curve on the negative side of the normal stress axis shows there is a limit to the amount of tension the material can take. In this case ϕ would equal to zero degrees.

Failure angle ϕ is equal to $\frac{\pi}{4} - \frac{\theta}{2}$ where θ is the angle of internal friction so that this theory predicts a shear failure at a stress equal to the shearing strength plus the normal stress times the tangent of θ . In Coulomb's theory θ is constant.

To prove that intermediate stress has no influence we have to make tests by using small axial stress and larger lateral pressure. In this case we should get exactly the same limiting curves. The author has not done any tests of this kind. Bulletin 185 of the University of Illinois Engineering Experiment Station presents results of some tests of this kind. These tests indicated lower failing values when axial stress was less than lateral pressure than occurred when axial stress was greater than lateral pressure. This is the opposite result of Kármán and Böcker's tests on marble. It was concluded in the Illinois bulletin that the tests were not sufficient to prove or disprove the point in question.

The greatest advantage of Mohr's theory is that we can obtain the

angle of sliding from it as mentioned before. By comparing it with the actual surfaces of failure deviation from a pure shear could be seen. Considering Figure 25 we see that the angle of failure changes from 42° in group E (richest mixture) to 37° for group I (poorest mixture). And there is an average of 7° difference between tests of highest lateral pressure and no lateral pressure. We can conclude by using Professor Terzaghi's term that failure forunjacketed specimens are pseudo-shear type or a failure between a splitting and a shear type, as actual angles of failure are lower than these values.

As for the jacketed specimens the failure angle on Mohr diagrams changes between 33° and 29° and there is not considerable difference between individual mixtures. This fact is consistent with the University of Illinois results. We can conclude that jacketed specimens usually failed close to a shear type failure.

Comparing Mr. Brandtzaeg's theory we see that he based his analysis on an ideal material of non-isotropic elements. Its difference from Mohr's theory is that instead of the material itself its elements yield plastically through intragranular sliding along planes of one direction fixed for one element but varying from one element to the other. Mr. Brandtzaeg introduces the well-known Coulomb equation for elements, not for the whole material, which defines the plastic equilibrium of the elements. Mr. Brandtzaeg also assumes that plastic elements deform proportionally to the stresses they carry plus a plastic sliding so that their total deformation is equal to that of elastic elements which results in all elements deforming alike. An element will not be free to deform in its own direction of weakness unless it breaks off.

A point against this theory is that most of the constants still had to be determined experimentally. Also, Mr. Brandtzaeg disregards the pores which play the dominant part with materials that have pore pressure. If the voids are empty still there is a compacting of the material. In Mr. Brandtzaeg's equations unit volumetric deformation of the material is the same as in individual elements. The effect of voids on failure is obviously seen in the author's experiments. As different elements will reach plastic stages at different times this theory shows the passage of the material from the elastic to the plastic stage.

In simple compression plastic elements can carry stresses only if they are supported laterally by elastic elements, which produces a lateral tension in elastic elements. The same condition occurs in specimens subjected to lateral compression where this action is retarded and a general sliding and disorganizing occurs. Mr. Brandtzaeg concludes that the eventual failure still will be due to a limiting tension. His analysis does not apply after splitting starts.

Professor Terzaghi⁽⁷⁾ considers failures in simple compression as a splitting type but he does not account for it definitely as he says failure by splitting is not strictly comparable to failure by pure tension because the mutual displacement of the individual constituents which precedes these two types of failure is somewhat different. But nevertheless he says the material fails by splitting as soon as the negative scatter stress equals the tensile strength of the material. For the case of triaxial failure his analysis in basic principles corresponds to the author's. As seen from Fig. 3, with increasing confining pressure failure will depend more and more on the average stress, which is the lateral stress, though still the tensile strength will govern the failure

but with substantially more normal stress. The equation Professor Terzaghi derives (4) gives an increase in strength ten times the lateral pressure. Compared to author's 3.9 and University of Illinois' 4.1 that is too high. In finding stresses by Equation (5) Professor Terzaghi uses cohesive strength and includes the fact that most of the failures are intergranular though intragranular for high lateral pressures.

The author's way of relating the effect of pore pressure is basically the same as that used by Professor Terzaghi. A reduction formula is introduced to all of the formulas (see section on Previous Work).

Mr. McHenry⁽¹⁰⁾ approached the problem with the Mohr-Coulomb theory, so that there is no disagreement with the author in that respect, but his results about the effect of pore pressure does not agree with the author's. Reference to this will be made later. Also his increase in strength of material is 5.8 times the lateral pressure which does not agree with the author's 3.9.

Initial slope averages in tables 10 and 11 show a definite decrease with respect to their values in zero oil pressure. This is due to the effect of the oil pressure. Those are the initial slopes after oil pressure and some excess load was applied but before then the specimens also deflect due to an inelastic compacting in all directions. This point and the general appearance of the curves correspond to the University of Illinois results.

Volume changes for jacketed specimens show a definite decrease of volume due to inelastic compacting which is almost the same at the same lateral pressure for all groups except group J. It seems that added strength due to lateral pressure is the same for all groups except group J. This could be seen by the similarity of both load-deflection curves

and volume change curves. Group J shows larger decreases in volume and increase in deformation than all the rest. This is due to its having a higher porosity than the others (Fig. 5). This proves that pores affect the failure of material even when the voids are empty and should be considered.

In Figure 6 load deformation for group H are plotted, where deflections during the application of oil pressure are also included. This figure will clearly show the increase in strength in axial direction with increasing oil pressure. This is done by applying a correction of 5×10^{-4} inches per 2500 psi oil pressure (same as early stages of loading) to Column 6 readings of data*. Final deflection, when we reach constant oil pressure, is added to each Column 8 reading to get deformations at various axial loads.

The effect of lateral pressure on the strength of the specimen is represented by a polynomial of the form:

$$Y_1 = y_c + b Y_2 + c Y_2^2$$

where Y_1 is the maximum axial load, Y_2 is the lateral pressure, and y_c is the standard compressive strength of the concrete. An equation of this type could be written for each experiment, from which coefficients b and c could be determined. These equations for jacketed and unjacketed specimens should be written separately as they are tested under different conditions. As we have more equations than unknowns, the author used the method of least squares to solve them. This method is explained on the following page.** The resulting equation for the jacketed specimens:

$$Y_1 = y_c + 4.5 Y_2 - 0.56 \times 10^{-4} Y_2^2 \quad (9)$$

* See Appendix II.

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Group	Change in Volume	
	* grams	** porosity in per cent
D	0.5	15.4
F	0.68	21.1
H	0.6	18.8
J	1.0	31

* These are the averages of weight decreases, for each group, after moisture control.

** A sample calculation for group J is presented below.

$$\text{Change in Volume} = 1 \text{ gr} = 2.2 \times 10^{-3} \text{ lb}$$

$$V = \frac{2.2 \times 10^{-3} \text{ lb}}{62.4 \text{ lb/ft}^3} = 0.352 \times 10^{-3} \text{ ft}^3 \text{ or } 0.352 \times 10^{-3} \times 1728 \text{ in}^3$$

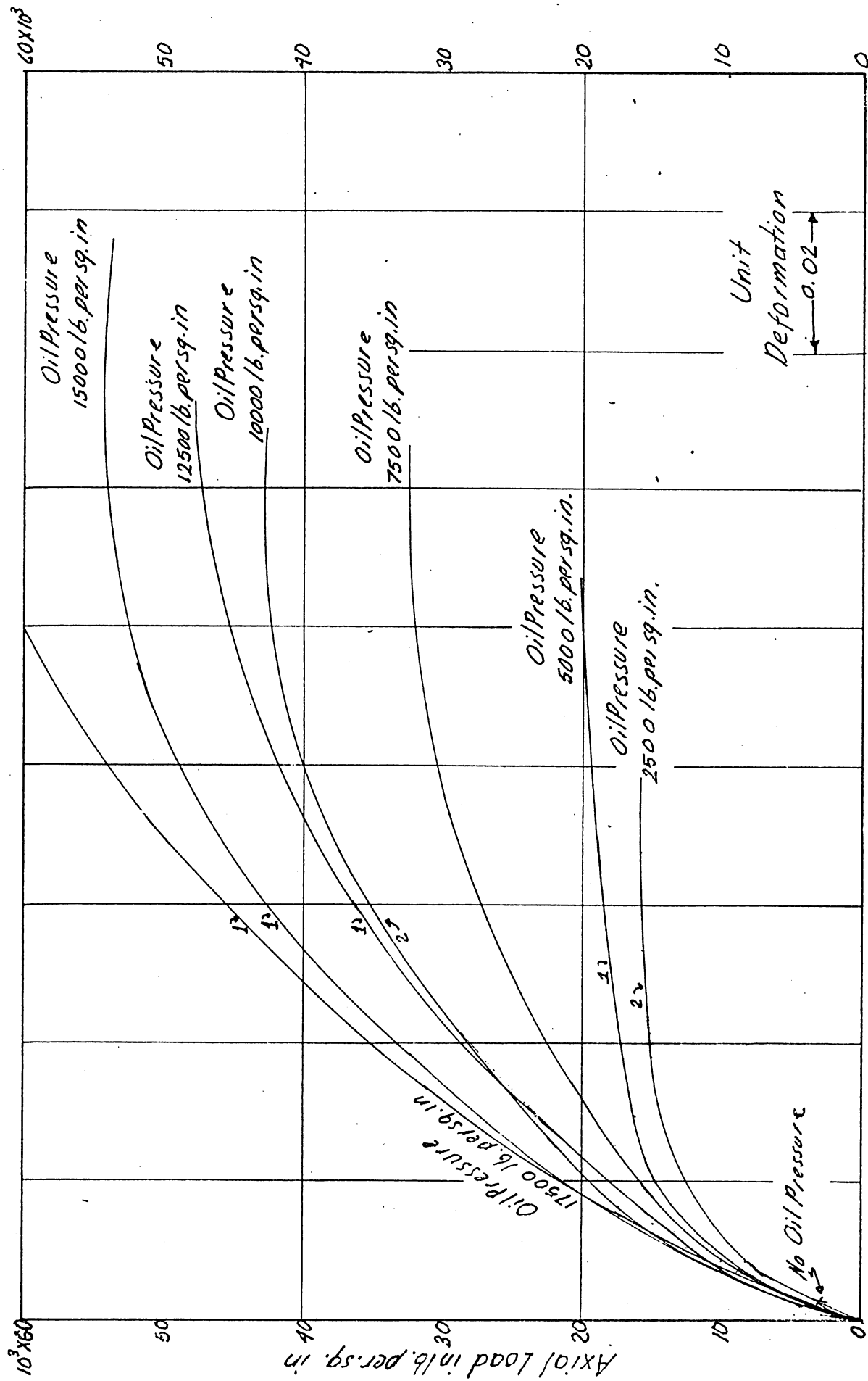
$= 0.061 \text{ in}^3$ This is the amount of water removed from the specimen.

Volume of a $\frac{1}{2}$ by 1 inch specimen is $\frac{\pi(\frac{1}{2})^2}{4} = 0.196 \text{ in}^3$
per cent change in volume:

$$\frac{.061}{.196} \times 100 = 31\%$$

This is the porosity in percent for Group J.

Figure 5



Unit Deformation in in. per in.
 Fig 6 Load Deformation Curves for Specimens with empty voids in triaxial Compression
 Group H

This equation represents the effect of the lateral pressure on strength in the axial direction. It is plotted in Figure 7. Experimental results are also shown on the same figure. As explained in the presentation of the results, increase in axial load with increasing lateral pressure follows a curve whose slope decreases with increasing lateral pressure. This fact is consistent with University of Illinois results. Equation (9) follows a similar pattern. The author has also drawn a straight-line relation in Fig. 8 which gives the following approximate equation for jacketed specimen:

$$Y_1 = y_c + 3.9 Y_2 \quad (10)$$

The 3.9 in this equation compares with a coefficient of Y_2 of 4.1 in the Illinois tests. As the author's tests were carried to higher lateral pressures than the Illinois tests a lower coefficient, i.e., a lower average slope of the line, would be expected in the author's results. The advantage of Equation 9 over 10 is that we get higher maximum axial loads at lower pressures as the effect of $0.56 \times 10^{-4} Y_2^2$ is very little at low lateral pressures. Equation 9 also represents experimental results better.

The resulting equations for the unjacketed specimens are as follows:

The quadratic equation:

$$Y_1 = y_c + 1.41 Y_2 - 0.11 \times 10^{-4} Y_2^2 \quad (11)$$

The straight-line equation:

$$Y_1 = y_c + 1.2 Y_2 \quad (12)$$

As mentioned in the results the same y_c is used for the jacketed and unjacketed specimens of each mixture as there is no obvious difference between the two. Equations 9 and 11 could be reduced into one equation which would represent the effect of lateral pressure at different

Given equations are:

$$Y_{11} - Y_c = (b)Y_{21} + (c)Y_{21}^2$$

$$Y_{12} - Y_c = (b)Y_{22} + (c)Y_{22}^2$$

$$Y_{1n} - Y_c = (b)Y_{2n} + (c)Y_{2n}^2$$

Where, b and c are the unknowns. The, n , given equations arranged in the following form:

$$[Y_2 Y_2]b + [Y_2 Y_2^2]c = [Y_2 (Y_1 - Y_c)]$$

$$[Y_2 Y_2^2]b + [Y_2^2 Y_2^2]c = [Y_2^2 (Y_1 - Y_c)]$$

These equations are solved to get best values of a and b .

Notations: $[Y_2 Y_2] = Y_{21}^2 + Y_{22}^2 + Y_{23}^2 + \dots + Y_{2n}^2$

$$[Y_2 Y_2^2] = Y_{21}^3 + Y_{22}^3 + Y_{23}^3 + \dots + Y_{2n}^3$$

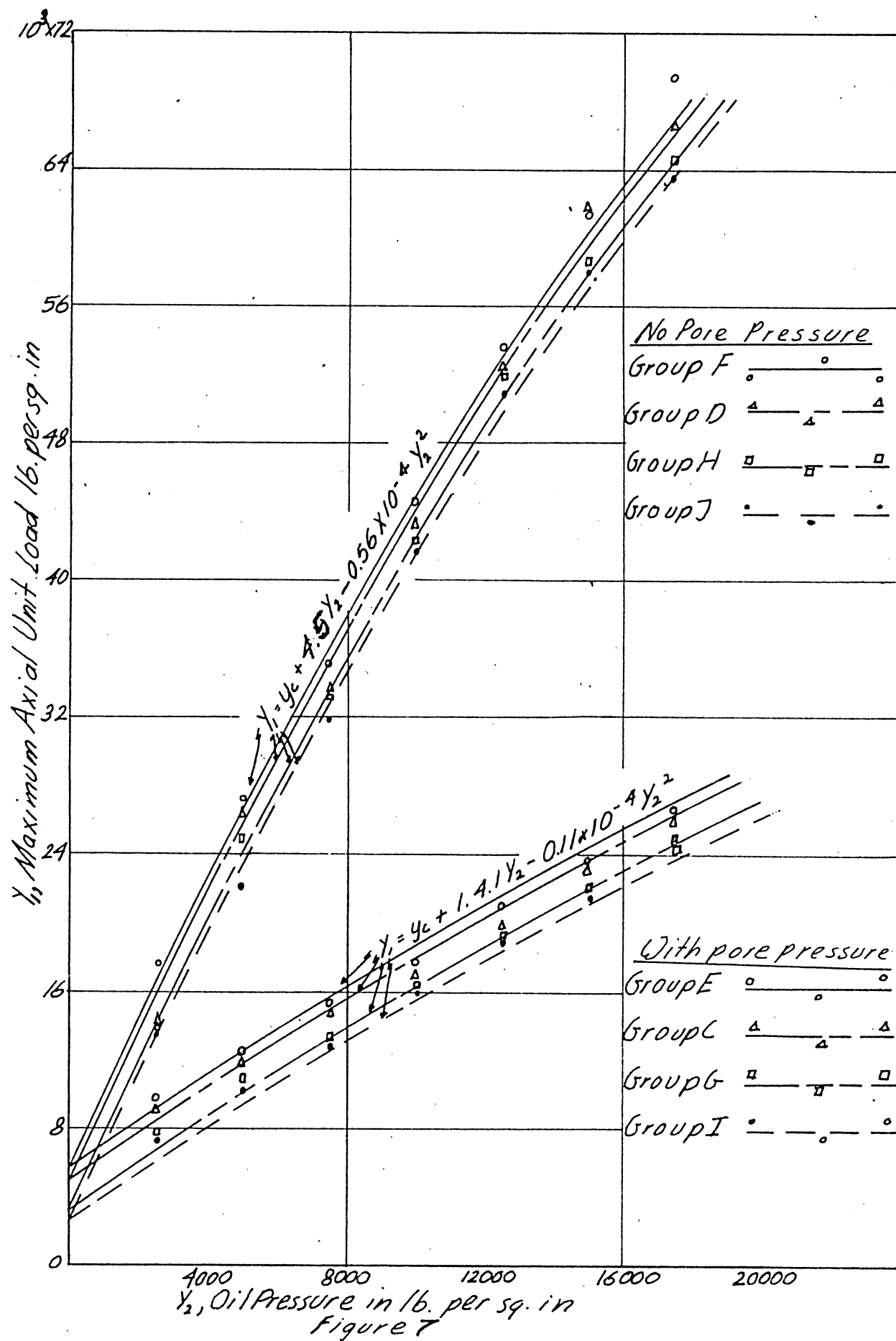
$$[Y_2^2 Y_2^2] = Y_{21}^4 + Y_{22}^4 + Y_{23}^4 + \dots + Y_{2n}^4$$

$$[Y_2 (Y_1 - Y_c)] = Y_{21}(Y_{11} - Y_c) + Y_{22}(Y_{12} - Y_c) + \dots + Y_{2n}(Y_{1n} - Y_c)$$

Sample Calculation for jacketed specimens:

$$\begin{aligned} [Y_2 (Y_1 - Y_c)] &= 2500(17700 - 5620) + 5000(27200 - 5620) + 7500(35100 - 5620) \\ &+ 10000(44500 - 5620) + 12500(53500 - 5620) + 15000(61100 - 5620) + \\ &17500(69300 - 5620) + 2500(14300 - 4870) + 5000(26500 - 4870) + \\ &7500(35000 - 4870) + 10000(43000 - 4870) + 12500(52500 - 4870) + \\ &15000(61600 - 4870) + 17500(66500 - 4870) + 2500(14300 - 3160) + \\ &5000(24800 - 3160) + 7500(33600 - 3160) + 10000(42000 - 3160) + \\ &12500(50900 - 3160) + 15000(58500 - 3160) + 17500(64500 - 3160) + \\ &2500(13800 - 2480) + 5000(21900 - 2480) + 7500(31600 - 2480) + \\ &10000(41600 - 2480) + 12500(50600 - 2480) + 15000(58000 - 2480) + \\ &17500(63400 - 2480) = \underline{1,303,700 \times 10^4} \end{aligned}$$

Average maximum axial load Y_1 is used at each oil pressure for each mixture. (Tables 9 and 10)



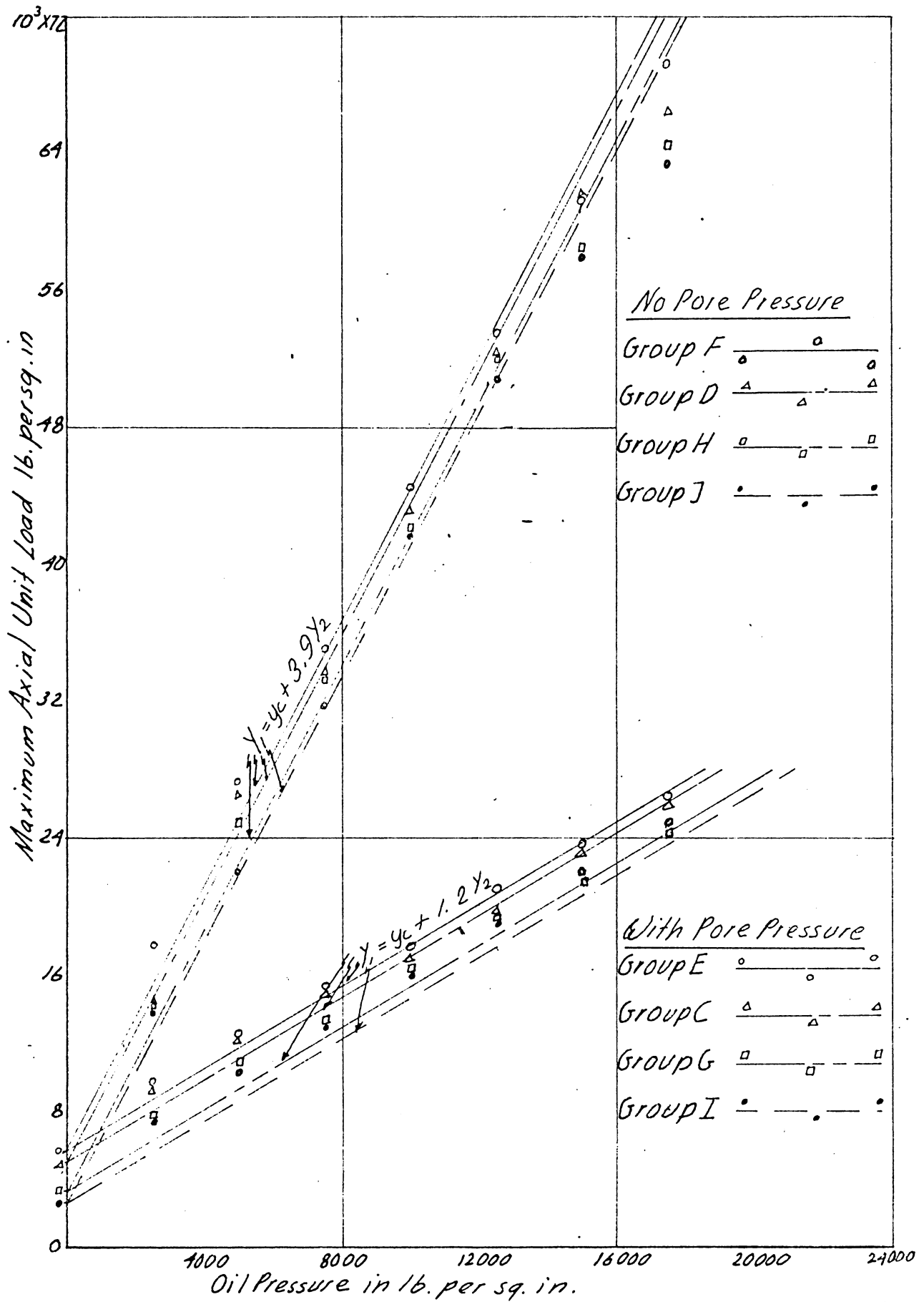


Figure 8

pore pressures P:

$$Y_1 = y_c + 1.41 P - 0.11 \times 10^{-4} P^2 + 4.5(Y_2 - P) - 0.56 \times 10^{-4}(Y_2^2 - P^2) \quad (13)$$

The second part of the experiments, the unjacketed tests, was done to find the effect of the pore-water pressure on the strength of the specimens. This effect will be a reduction in the lateral support of the material. As pore pressure will counteract the lateral stresses, there will be a decrease in the strength of the material in the axial direction depending on the effectiveness of pore pressure P. Accordingly Equation 9 should reduce into Equation 11 if we deduct effective pore pressure from lateral stresses.

$$Y_1 = y_c + 4.5 (Y_2 - P n_a) - 0.56 \times 10^{-4} (Y_2^2 - P^2 n_a^2) \quad (14)$$

or remembering that $Y_2 = P$

$$Y_1 = y_c + 4.5 Y_2 (1 - n_a) - 0.56 \times 10^{-4} Y_2^2 (1 - n_a^2) \quad (15)$$

where n_a is the reduction factor that will reduce equation 15 into 11. An $n_a = 0.69$ will reduce 4.5 into 1.41 and an $n_a = 0.90$ will reduce 0.56×10^{-4} into 0.11×10^{-4} . This way we get two different reduction factors where actually there should be one. As the author arrived at those two equations independently and also remembering the size of the specimens this discrepancy would seem justified. Since at low oil pressures the effect of the second degree term is negligible, it would be best to use 0.69 for n_a . The author suggests Equation 14 with an n_a factor of 0.69 for all cases, i.e., full pore pressure, no pore pressure, or various degrees of pore pressure.

This way we get the reduction factor which is part of the uplift

factor or the effect of the pore-water pressure on the strength of the material. In this analysis the author got the reduction factor in a purely experimental way, which should be the case as concluded in all the latest theories, contrary to the earlier ones. It is determined as a purely physical problem. It is an empirical constant that we should know, since in design of hydraulic structures it would be foolish either to disregard it or to consider a 100% effect which would make the design uneconomical.

When the voids are empty a lateral pressure provides an additional support to the bonds between particles, but when there is pore pressure the strength is reduced as the supporting pressure applied to the bonding material is reduced. Professor Terzaghi calls the reduction factor boundary porosity which he defines as the ratio of the part of the area of the potential surface of failure which is in contact with the interstitial liquid to the total area of this surface. This term of potential surface of failure is important as failure never occurs in a plane section contrary to Professor Fillunger's approach. Failure usually occurs in the binding material but as it is not a plane section it could be best determined experimentally by tests of failure, as we really do not know either the actual shape of the pores or the shape of the potential shape of failure.

In our experiments, specimens being small compared to the amount of hydraulic pressure applied, either the effect of additional weight due to water in the pores or the increase in hydraulic pressure due to its own weight with increasing depth is completely insignificant. But in hydraulic structures the weight of the water in the pores increases the specimen's own weight and thus reduces the uplift factor. This is

related to the quantity called effective volumetric porosity, n_v , of the material. It could be best determined experimentally. It is the amount of water that the material could absorb over the permanently present water content. A value of 7% is found by Mr. Leliavsky as an average value of permanently present water for the specimens he exposed to the sun in order to determine it. Now the final formula for uplift is

$$u_f = n_a - n_v$$

By multiplying this by the area of the pressure diagram we get the amount of uplift.

All the earlier theories considered the uplift factor equal to the average porosity of the material

$$u_f = n$$

That would give a value of 22%, taking the average porosity from Figure 5. They thought of it as a result of imaginary cracks. This is the average porosity of the material and does not agree either with the author's approach or coefficient.

Fillunger, who introduced the modern theory, made the plane section a basis. But we cannot do this as the material is not homogeneous and the failure most probably would not happen that way as explained before. His formula gives an uplift factor:

$$u_f = n_1 - n$$

where n_1 is the porosity of the mortar that binds the aggregates.

*Professor Terzaghi's approach gives a value of 0.78.

$$u_f = n_a - n$$

As seen from the above equation his reduction factor is 1.00 which is far from agreement with the author's but also his experimental values of 0.83 up to 1.20 are debatable too. Another difference is his using volumetric

*Refer to section on Previous Work

porosity instead of effective volumetric porosity. Effective volumetric porosity better expresses the true conditions as the material has always some amount of water present in its pores.

Mr. Leliavsky's formula for the uplift factor forms the last link in the modern theories which is:

$$u_f = n_a - n_v$$

His values for this equation were

$$u_f = 0.91 - 0.07 = 0.84$$

Mr. Serafim⁽¹²⁾ obtained values for uplift factor between 0.40 and 0.80.

Mr. McHenry found uplift factors ranging from 0.78 to 1.18 with an average of 1.02. Those values seem too high. His using gas for confining pressure probably influenced the results as it has a lower viscosity than water.

The author's reduction factor of 0.69 is less than the values obtained by others as explained. That might be partly due to his using oil as a medium for lateral pressure which has a higher viscosity than water which would make it harder for its penetration into finer voids. Also, in the unjacketed specimens some values of ultimate load deviated as much as 20% from average values. In the University of Illinois tests the maximum deviation was 11.8%, but a greater deviation might be expected in the author's results because of the small size of specimen. These factors suggest that possibly the value of reduction factor should be higher than 0.69, but perhaps not as high as Professor Terzaghi's and Mr. McHenry's values. The author also got a second n_a of 0.90. This too suggests a value higher than 0.69. In all the experiments on

the effect of pore pressure, the author believes that Mr. Lelivsky did the best one. He created the actual failure conditions that might happen in an hydraulic structure, viz., a constant weight producing compression, and a tension due to hydraulic pressure in the pores that might cause the failure.

CONCLUSIONS

Concrete may fail in one of two ways, brittle or ductile. In a brittle failure stresses are proportional to deformations almost up to fracture. In a ductile failure small changes of stress may be accompanied by large deformations; this happens between the elastic range and fracture.

The tests of jacketed specimens definitely prove that concrete subject to lateral compression shows a ductile behavior under axial compression, and it resists high stresses and shows large amounts of deflections, sometimes even up to $3/4$ th of its length. The relationships of stresses are given accurately by Mohr's theory. The comparison of angles suggests that shear plays a dominant part in failure. There is probably some splitting but mostly disorganizing of material and an intragranular type of failure due to sliding at an inclined direction. As suggested by Brandtzaeg the plastic elements are supported by elastic ones and that causes a tension in them, but under triaxial loading that action is retarded and its effect is much less important.

For specimens subject to pore pressure the condition of failure could only be expressed by Mohr circles using effective pore pressure.

There is an inelastic reduction in volume of concrete specimens with empty voids. The load-deflection curves and volume change curves are similar for all mixtures except one which has a much higher porosity than the rest. This proves the influence of voids on strength of a material whether the voids are empty or not. Group J with a greater porosity than any other mixture showed a larger decrease in volume and greater deformation than the rest of the mixtures.

The specimens with pore pressure in the voids failed in a brittle manner. Splitting due to lateral tension played some role in this as sliding occurred on one plane eventually.

There is no question that uplift force may exist in hydraulic structures and may cause their failure in tension if enough attention is not given to it. This force depends on the potential surface of failure, which is not a plane section, and also upon the ratio of the part of that failure area with which the liquid under pressure is in contact to the total of it. Due to these reasons it may be studied best by experimental means, such as was done in this study.

The relationship between stresses in triaxial compression, with or without pore pressure, could be defined by the following empirical equation:

$$Y_1 = y_c + 4.5(Y_2 - Pn_a) - 0.56 \times 10^{-4}(Y_2^2 - P^2n_a^2) \quad (14)$$

Y_1 = axial load at failure

Y_2 = the equal and smaller stresses, i.e., the confining lateral pressure

P = pore pressure

y_c = standard compressive strength of concrete

When there is no pore pressure this equation reduces to:

$$Y_1 = y_c + 4.5\frac{1}{2} - 0.56 \times 10^{-4} Y_2^2 \quad (9)$$

When pore pressure is equal to lateral pressure this equation reduces to:

$$Y_1 = y_c + 1.41\frac{1}{2} - 0.29 \times 10^{-4} Y_2^2 \quad (16)$$

The effect of pore-water pressure or boundary porosity could be expressed by the reduction factor, n_a , which has a value of 0.69.

This might be higher, as mentioned in the analysis. As can be seen, this ratio will reduce Equation 14 into 16. In hydraulic structures the effect of pore-water pressure is called uplift, where n_a represents only part of the pore pressure effect. The uplift factor is equal to:

$$u_f = n_a - n_v \quad (17)$$

Where n_v is the effective volumetric porosity of the material. Equation 17 is the factor that needs to be multiplied by the area of the pressure diagram in order to find the uplift force in hydraulic structures. n_v is the effective volumetric porosity which again needs to be found experimentally. It takes the permanently present water in the pores into consideration.

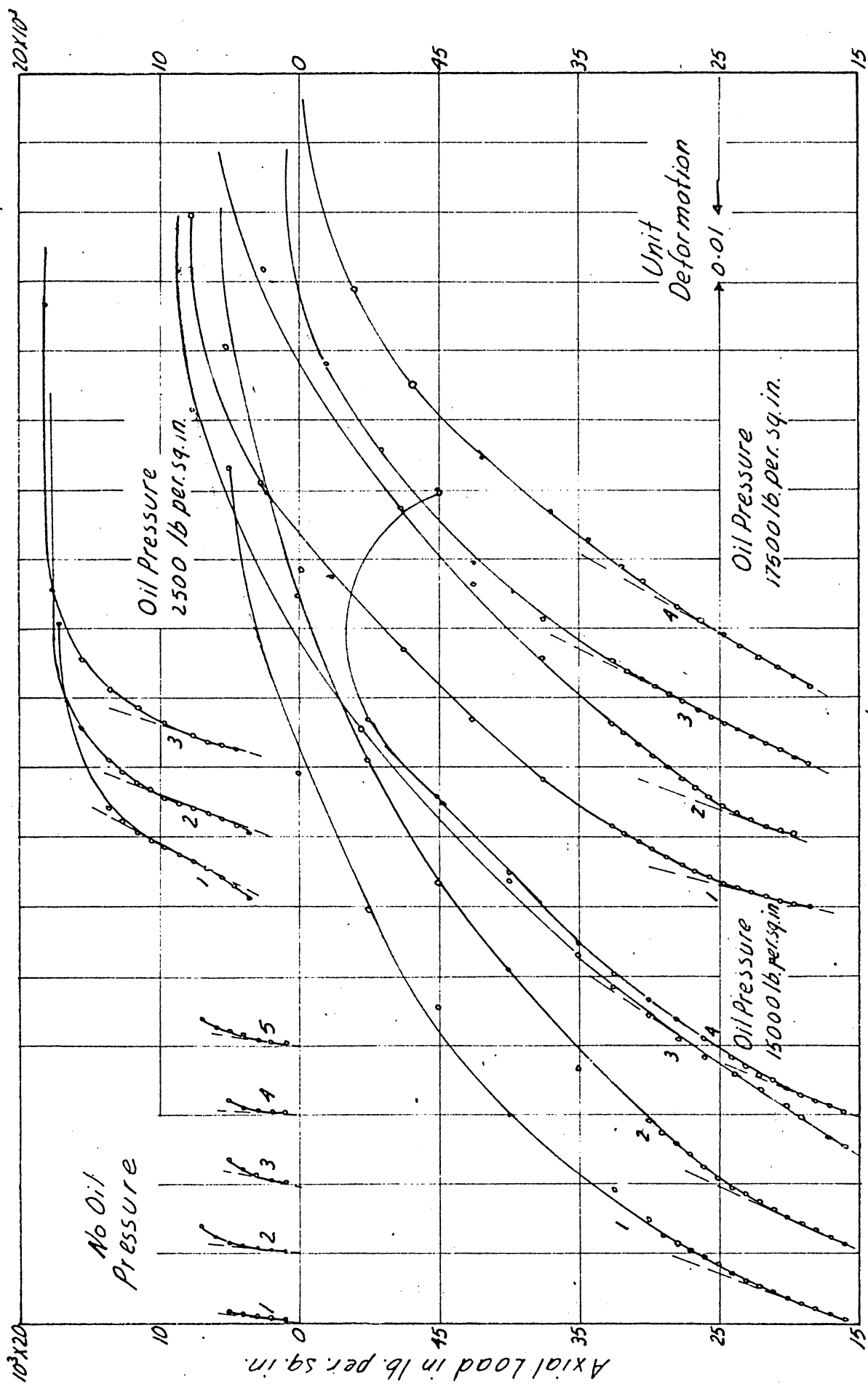
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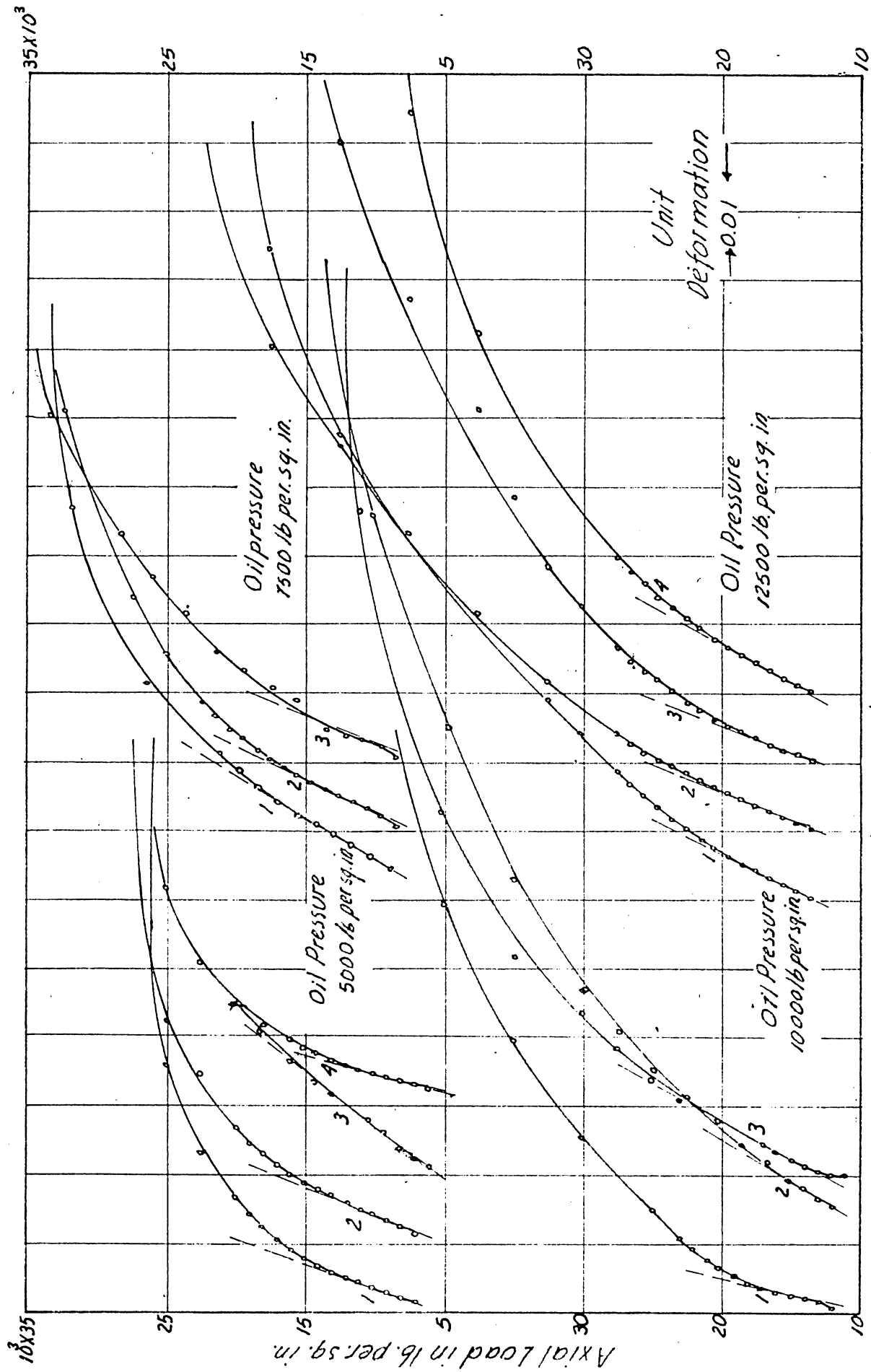
APPENDIX ILIST OF FIGURES IN ORDER

- Figure 9 & 10: Load Deformation Curves for group F, 1:2 Concrete, Tested Jacketed
- Figure 11 & 12: Load Deformation Curves for group D, 0.75: 0.92: 0.25 Concrete, Tested Jacketed
- Figure 13 & 14: Load Deformation Curves for group H, 1:3 Concrete, Tested Jacketed
- Figure 15 & 16: Load Deformation Curves for group J, 1: 1: 0.64 Tested Jacketed
- Figure 17: Load Deformation Curves for group E, 1:2 Concrete, Tested Unjacketed
- Figure 18: Load Deformation Curves for group C, 0.75: 0.92: 0.25 Concrete, tested unjacketed
- Figure 19: Load Deformation Curves for group G, 1:3 Concrete, Tested Unjacketed
- Figure 20: Load Deformation Curves for group I, 1: 1: 0.64 Concrete, Tested Unjacketed
- Figure 21 & 22: Volume Change Curves for group F, 1:2 Concrete, Tested Jacketed
- Figure 23 & 24: Volume Change Curves for group D, 0.75: 0.92: 0.25 Concrete, Tested Jacketed
- Figure 25 & 26: Volume Change Curves for group H, 1:3 Concrete, Tested Jacketed
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- Figure 29: Mohr Circle Diagrams for groups F & D
- Figure 30: Mohr Circle Diagrams for groups H & J
- Figure 31: Mohr Circle Diagrams for groups E, C, G, & I
- Figure 32: Schematic Diagram of the High Pressure Strength Test Apparatus
- Figure 33: Apparatus for Conducting Strength Tests on Rock under Hydrostatic Pressure in Place in Testing Machine

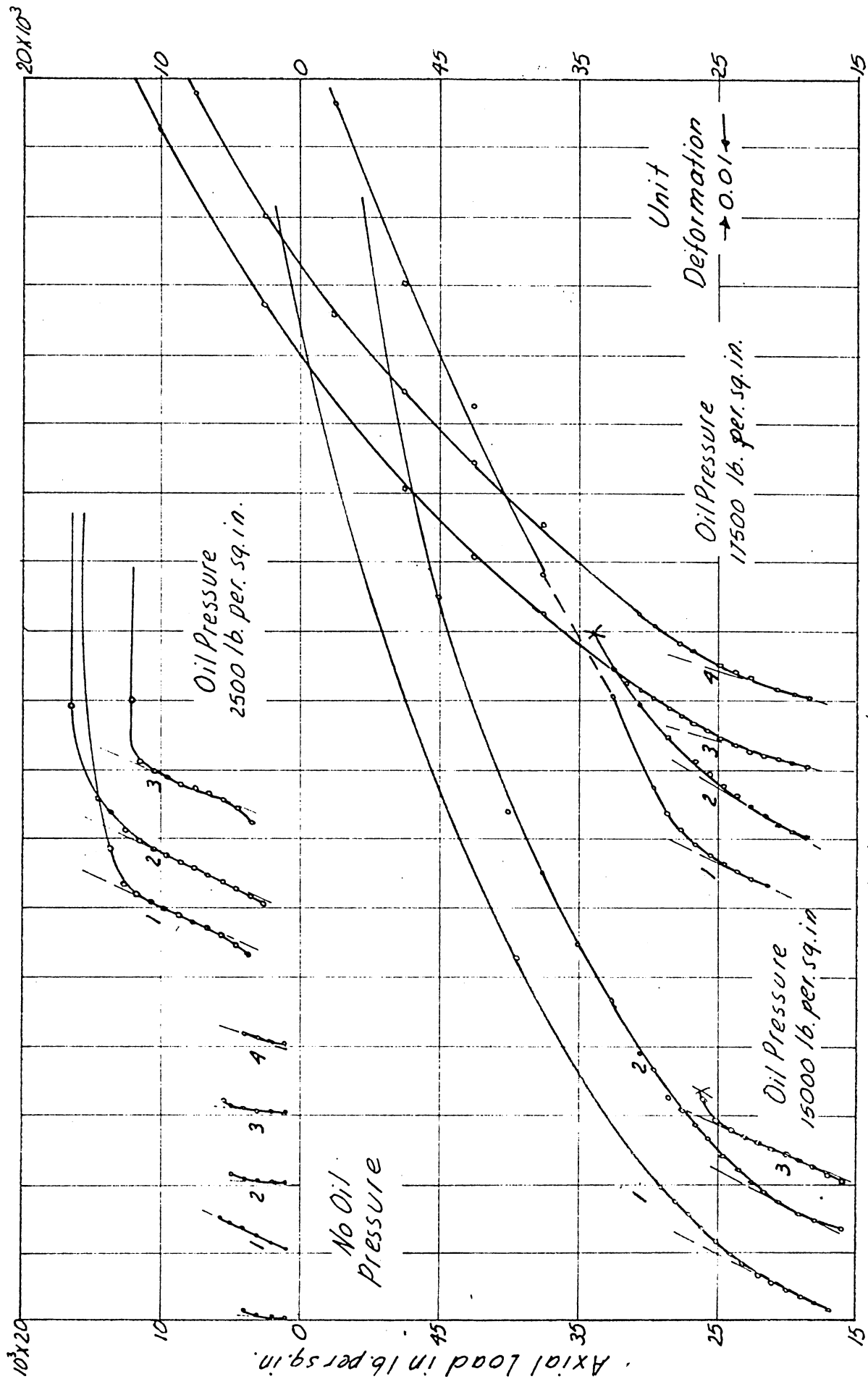
- Figure 34a: Specimen Assembly Showing Upper Piston, Lower Support, Brass Sleeves, and Plastic Tubing
- Figure 34b: 1: 2 Concrete, Tested Unjacketed at 10,000 psi Confining Pressure
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- Figure 34h: 1: 3 Concrete, Tested Jacketed at 12,500 psi Confining Pressure; Length = .270 inches; Diameter = 0.8.25



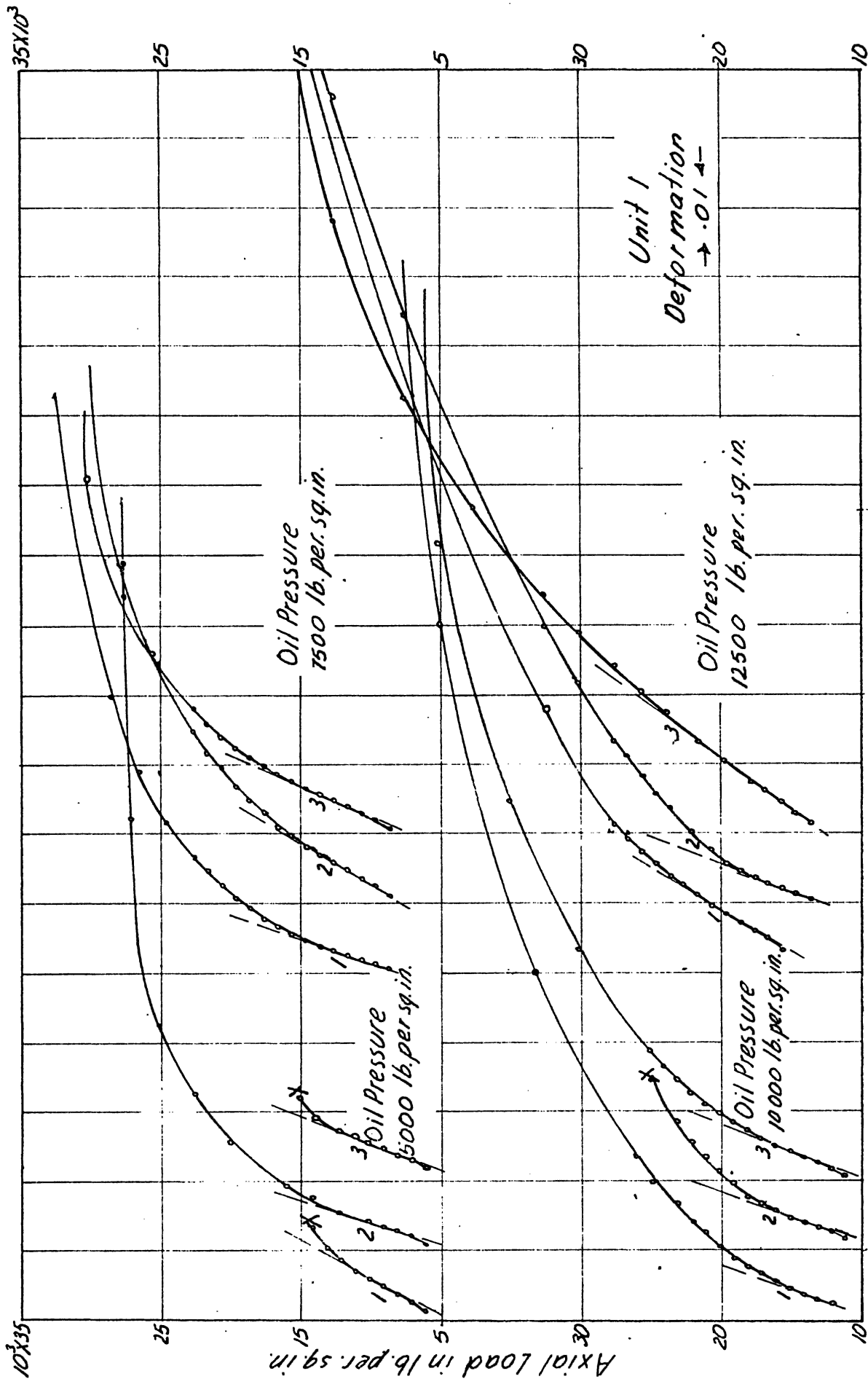
Unit Deformation in in. per in.
 Fig. 3 Load Deformation Curves for Specimens with empty voids in Triaxial Compression
 Group F



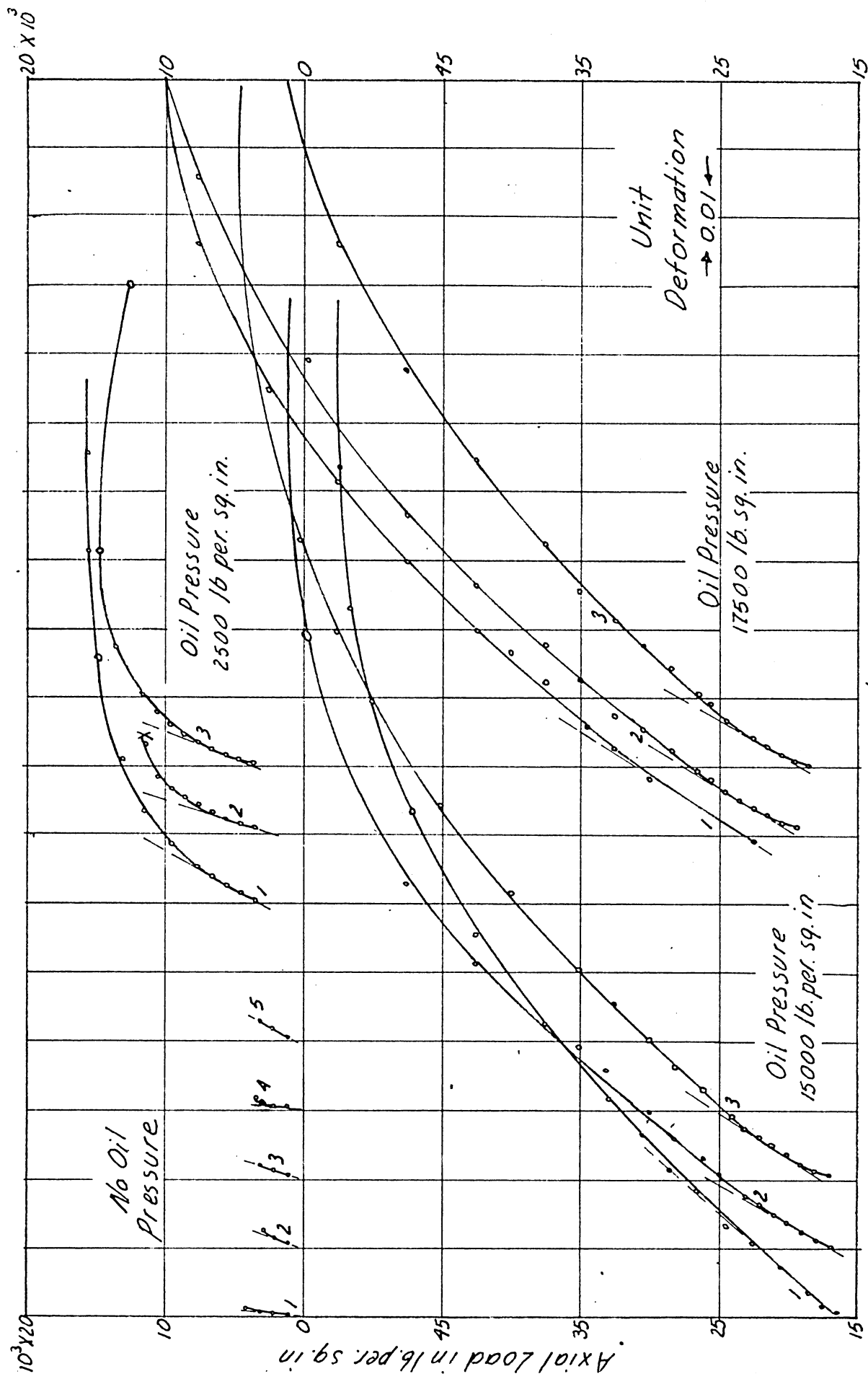
Unit Deformation in in. per in.
 Fig. 10 Load Deformation Curves for Specimens with empty voids in Triaxial Compression
 Group F



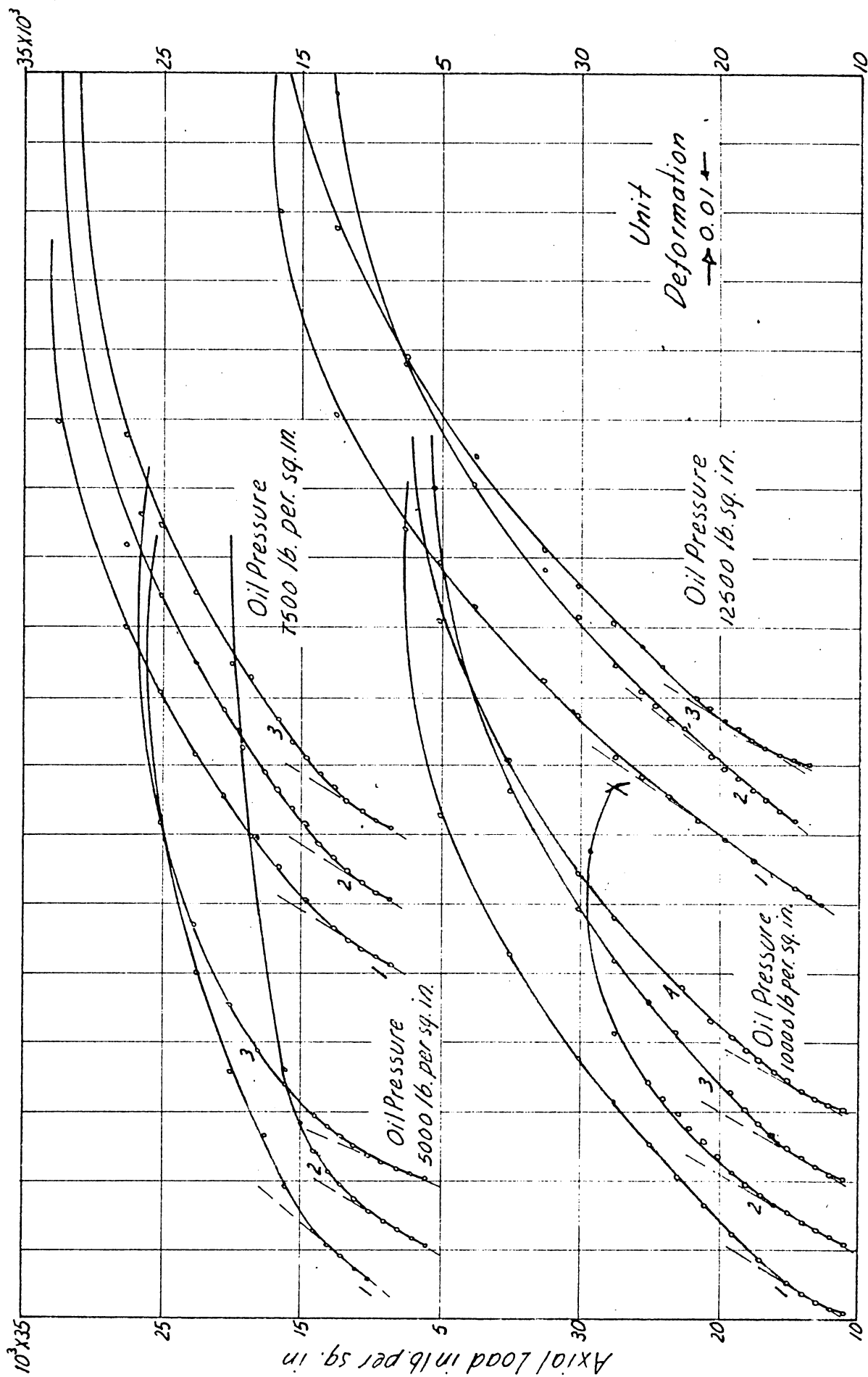
Unit Deformation in in. per in.
 Fig. 11 Load Deformation Curves for Specimens with empty voids in Triaxial Compression
 Group D



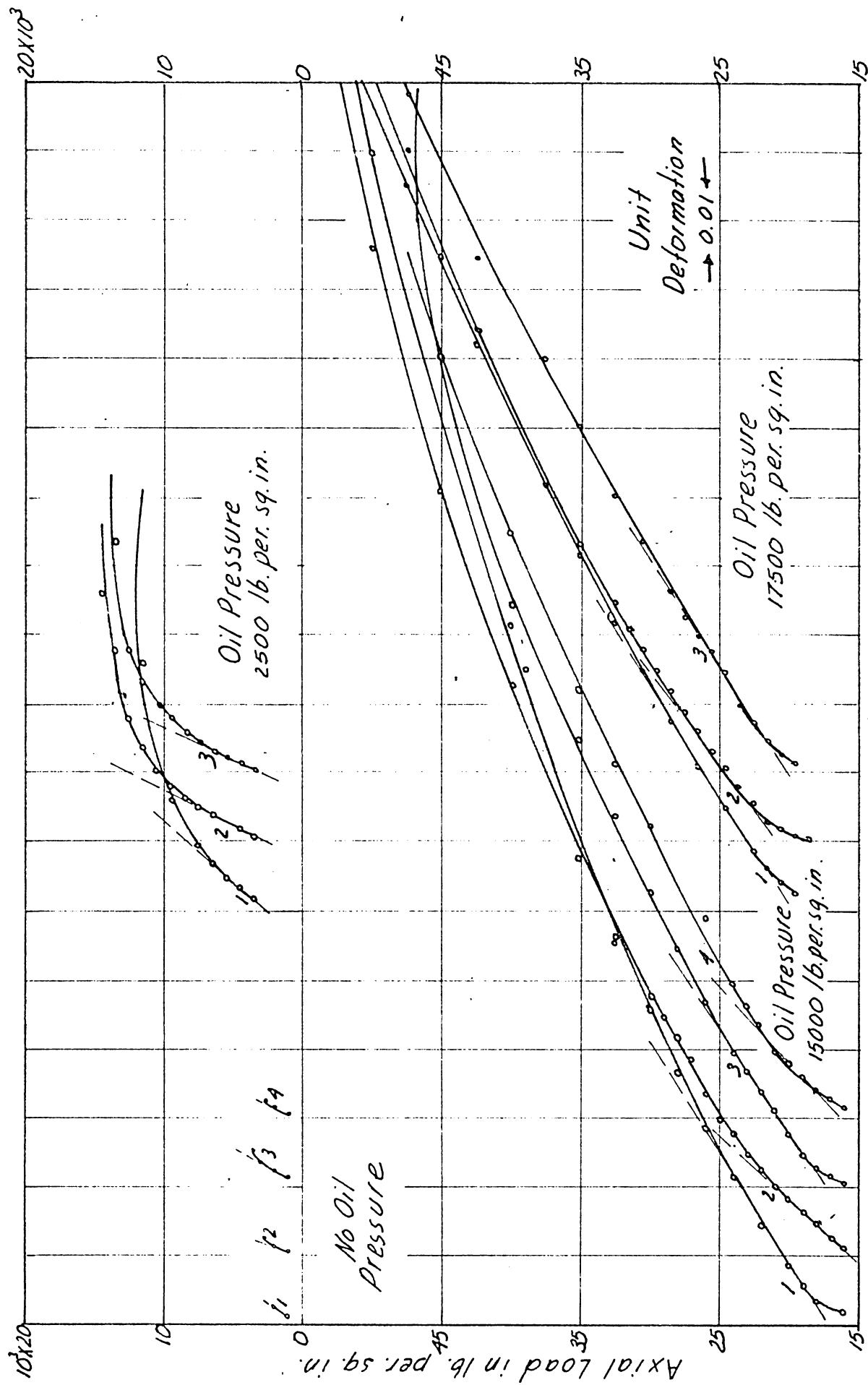
Unit Deformation in in. per in.
 Fig. 12 Load Deformation Curves for specimens with empty voids in triaxial compression
 Group D



Unit Deformation in. per in.
 Fig. 13 Load Deformation Curves for specimens with empty voids in Triaxial Compression
 Group H



Unit Deformation in in. per in.
 Fig 14 Load Deformation Curves for specimens with empty voids in triaxial Compression
 Group H



Unit Deformation in in. per in.
 Fig. 15 Load Deformation Curves for specimens with empty voids in triaxial Compression
 Group J

35×10^3

10×35

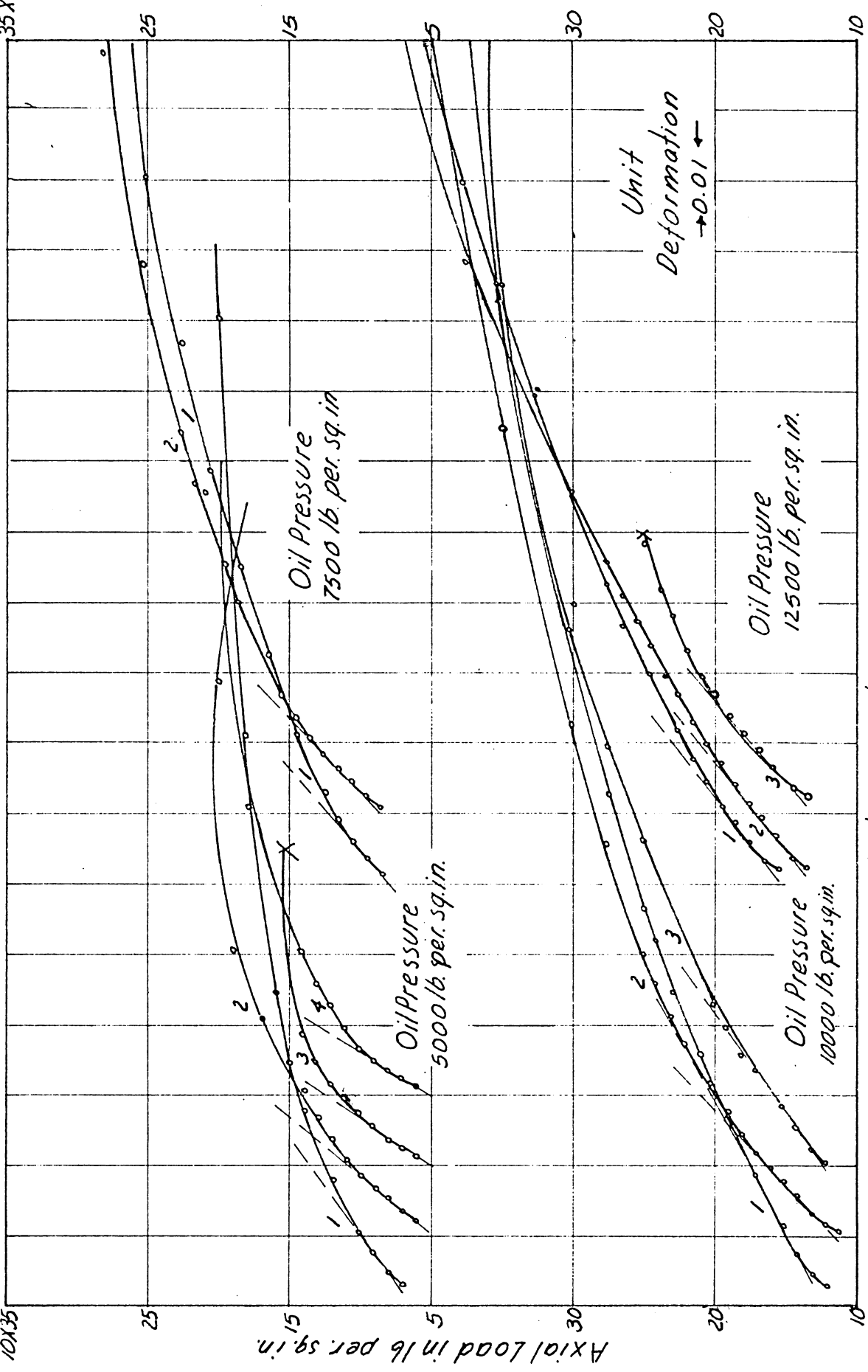


Fig. 16 Load Deformation Curves for specimens with empty voids in triaxial compression
Group J

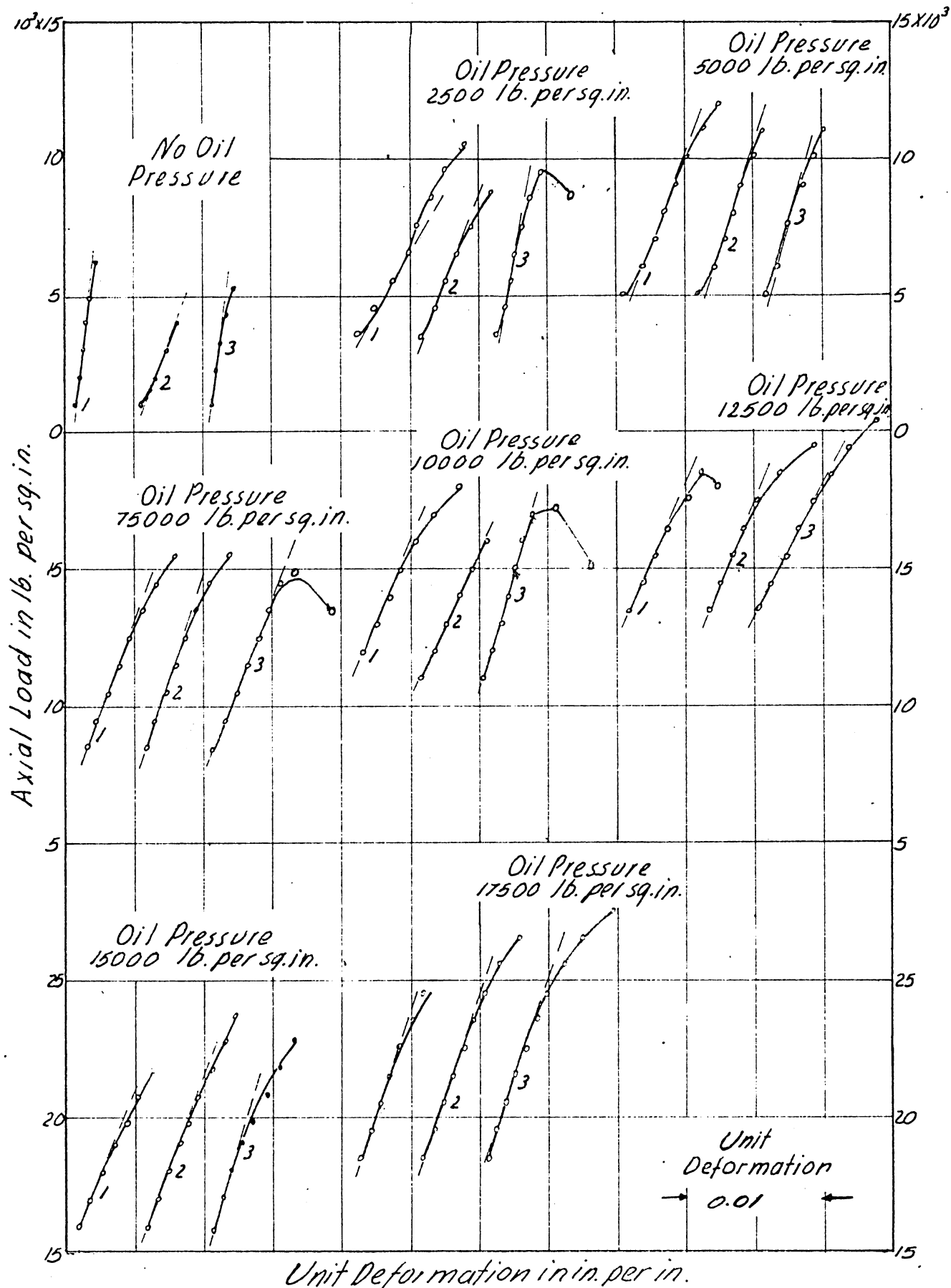


Fig. 18 Load Deformation Curves for Specimens in Axial Compression with pore pressures equal to oil pressures
Group C

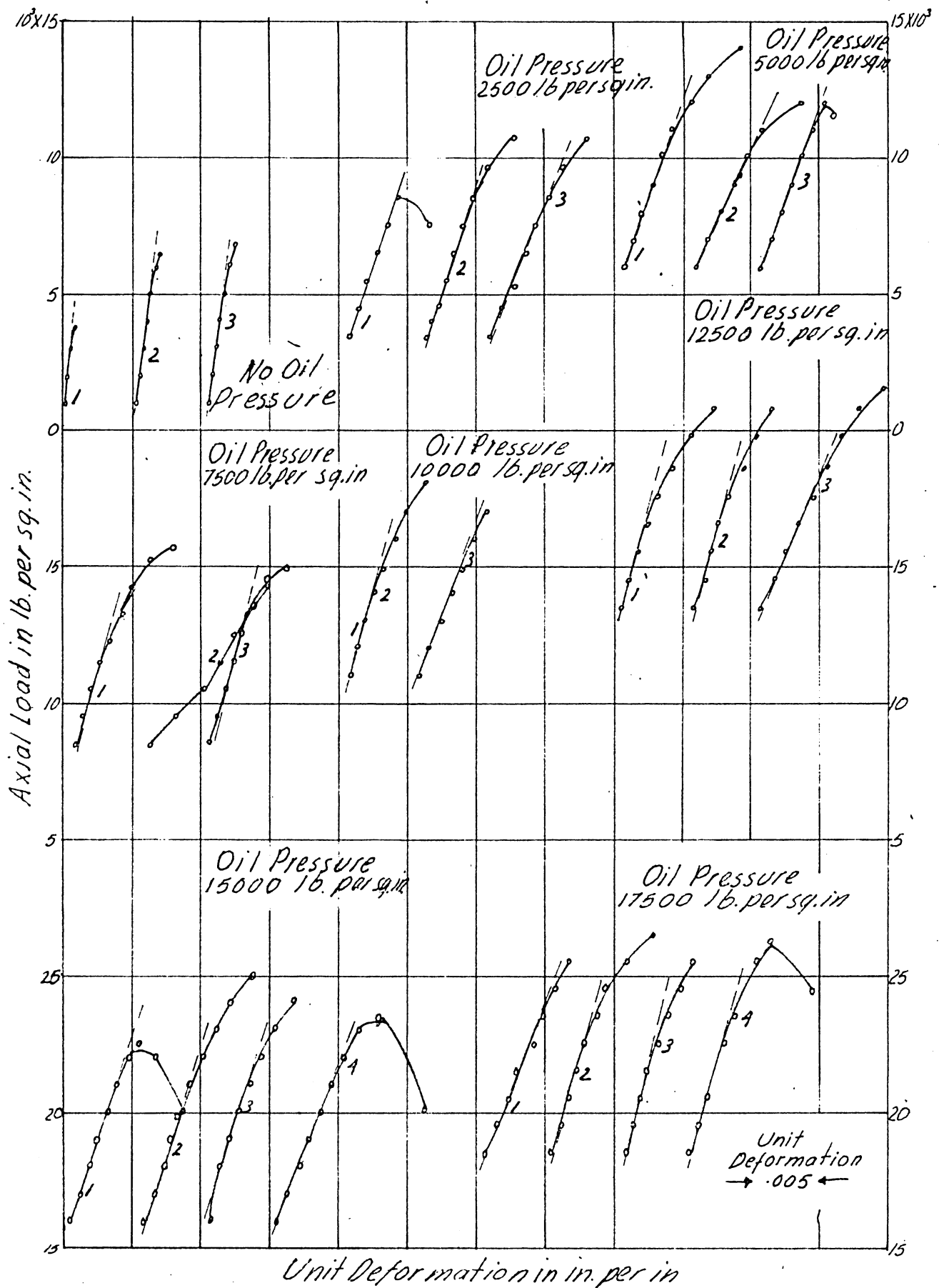


Fig. 17 Load Deformation Curves for Specimens in triaxial Compression with pore pressures equal to oil pressures
Group E

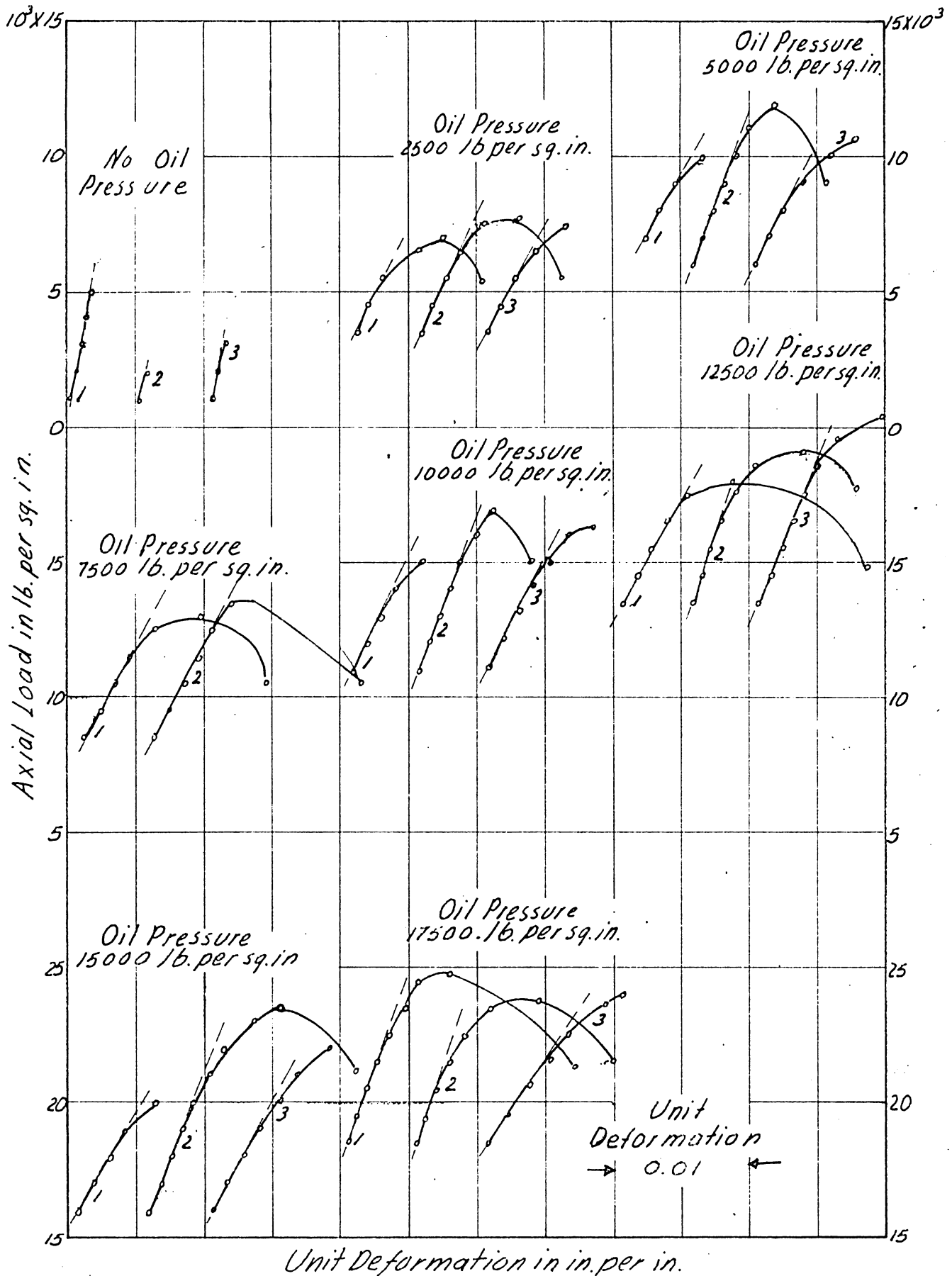


Fig. 19 Load Deformation Curves for Specimens in triaxial Compression with pore pressures equal to oil pressures
Group G

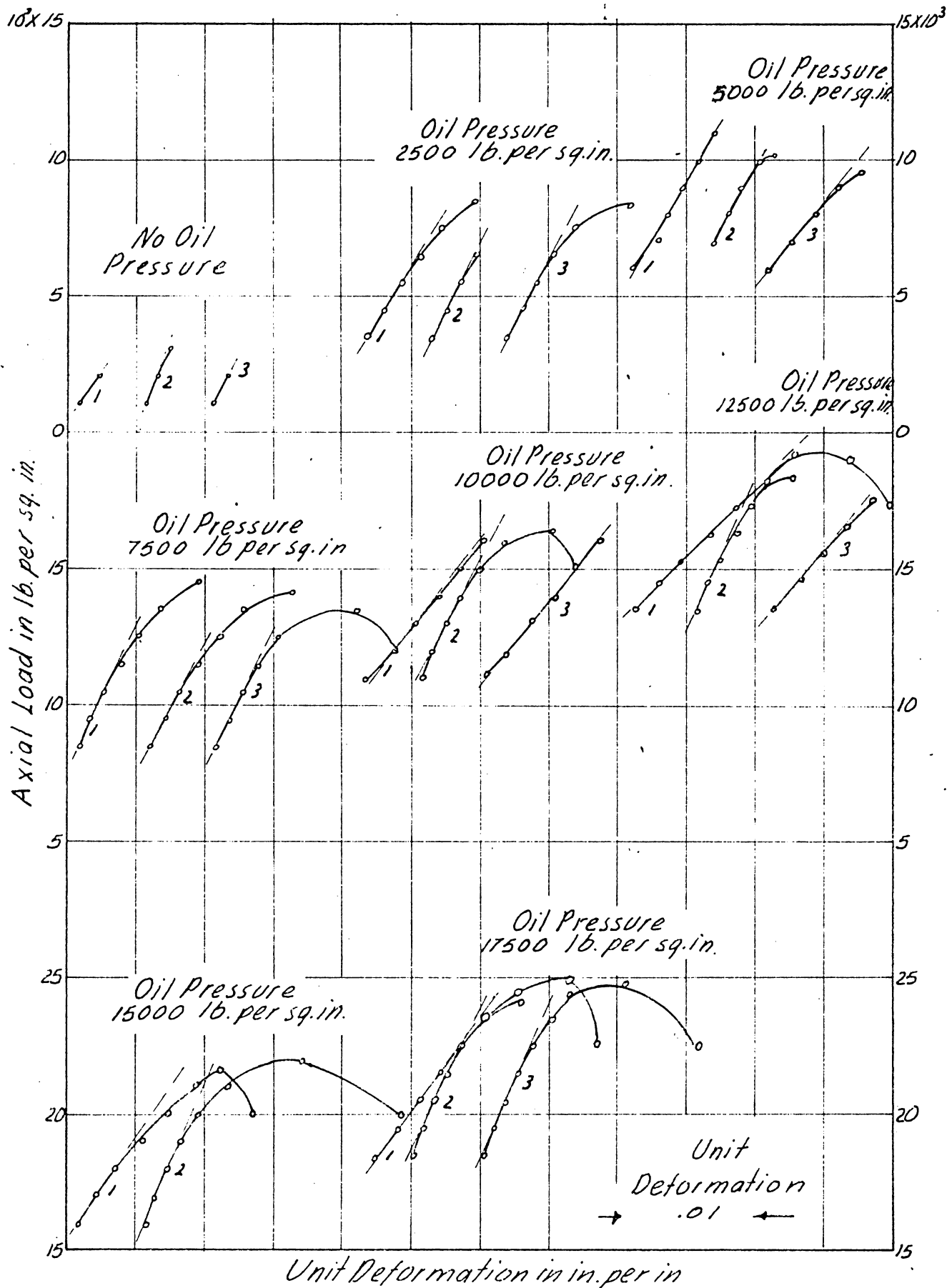
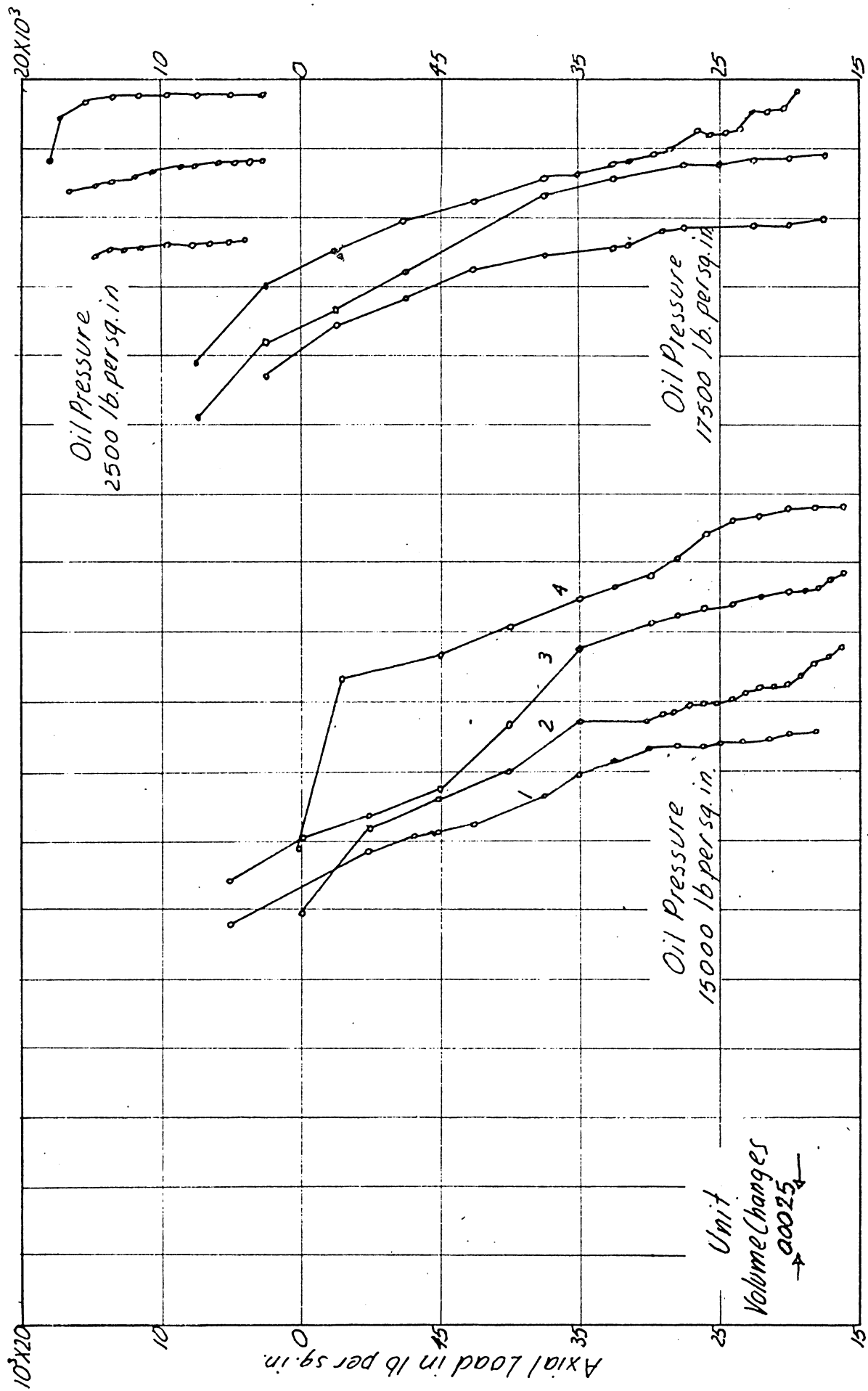
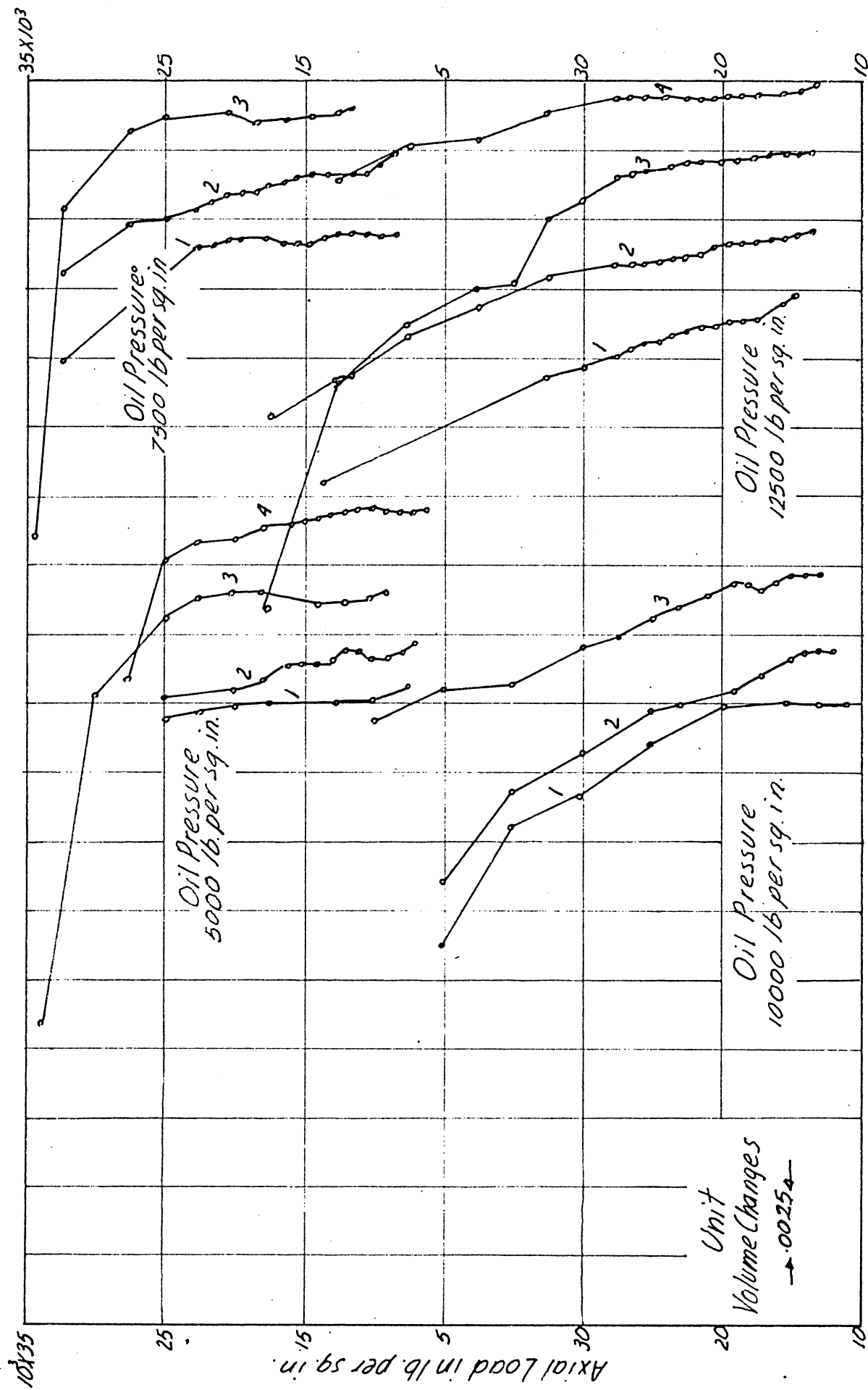


Fig. 20 Load Deformation Curves for Specimens in triaxial Compression with pure pressures equal to oil pressures
Group I



Unit Volume Change in in. cubes.
 Fig 21 Volume Change Curves for Specimens with Empty Voids in Triaxial Compression
 Group F



Unit Volume Change in in. cubes
 Fig 22 Volume Change Curves for Specimens with Empty Voids in Triaxial Compression
 Group F

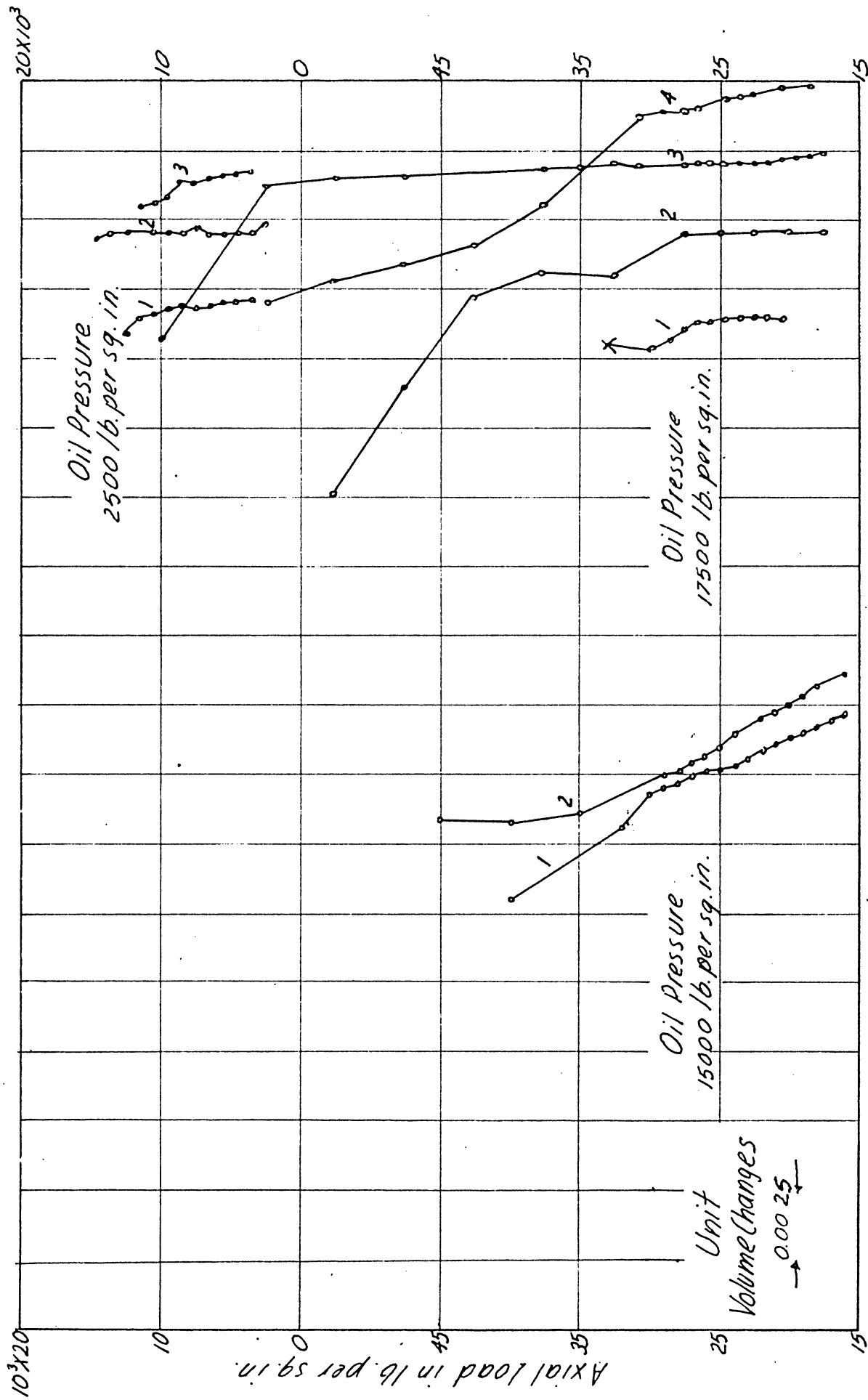
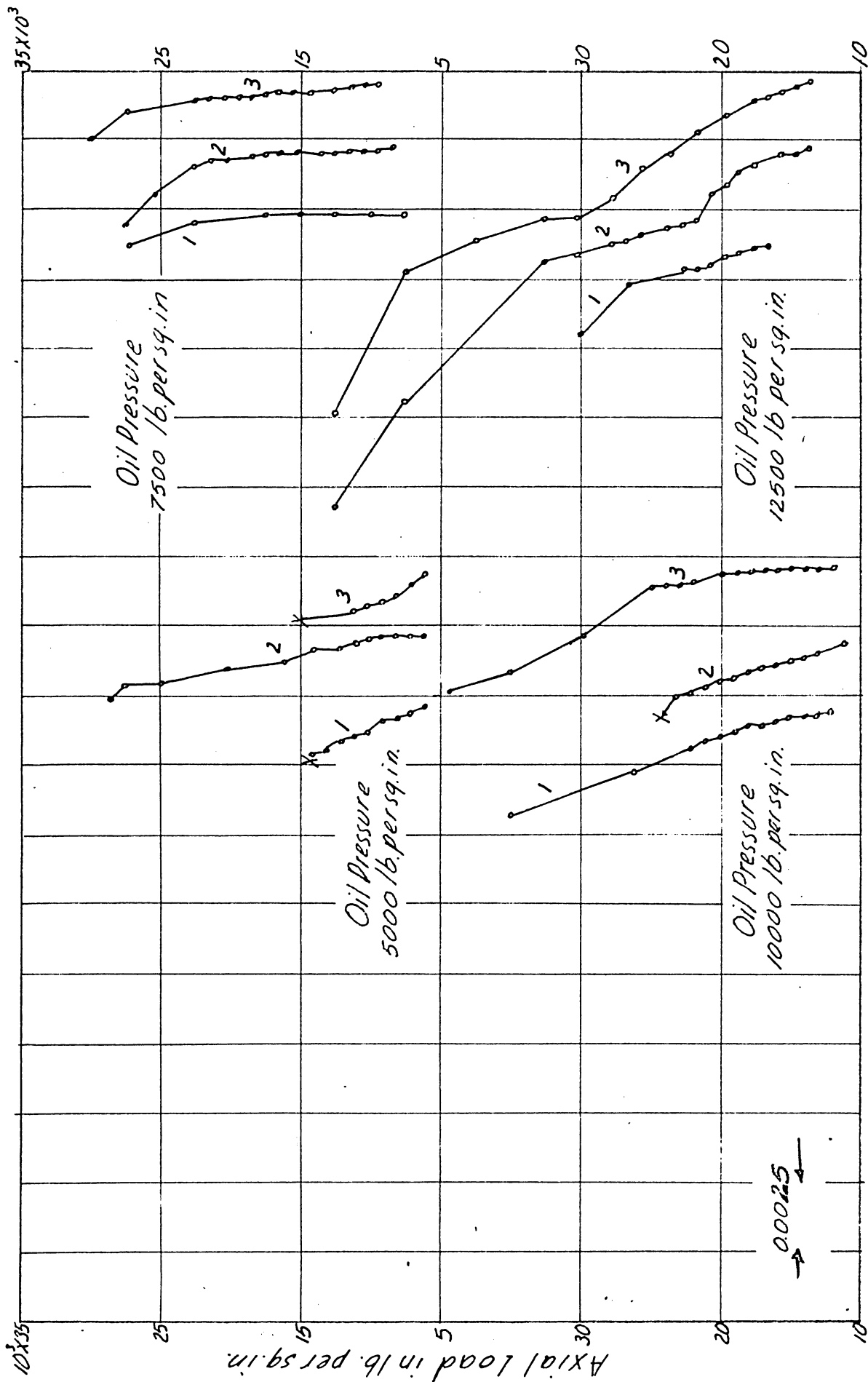
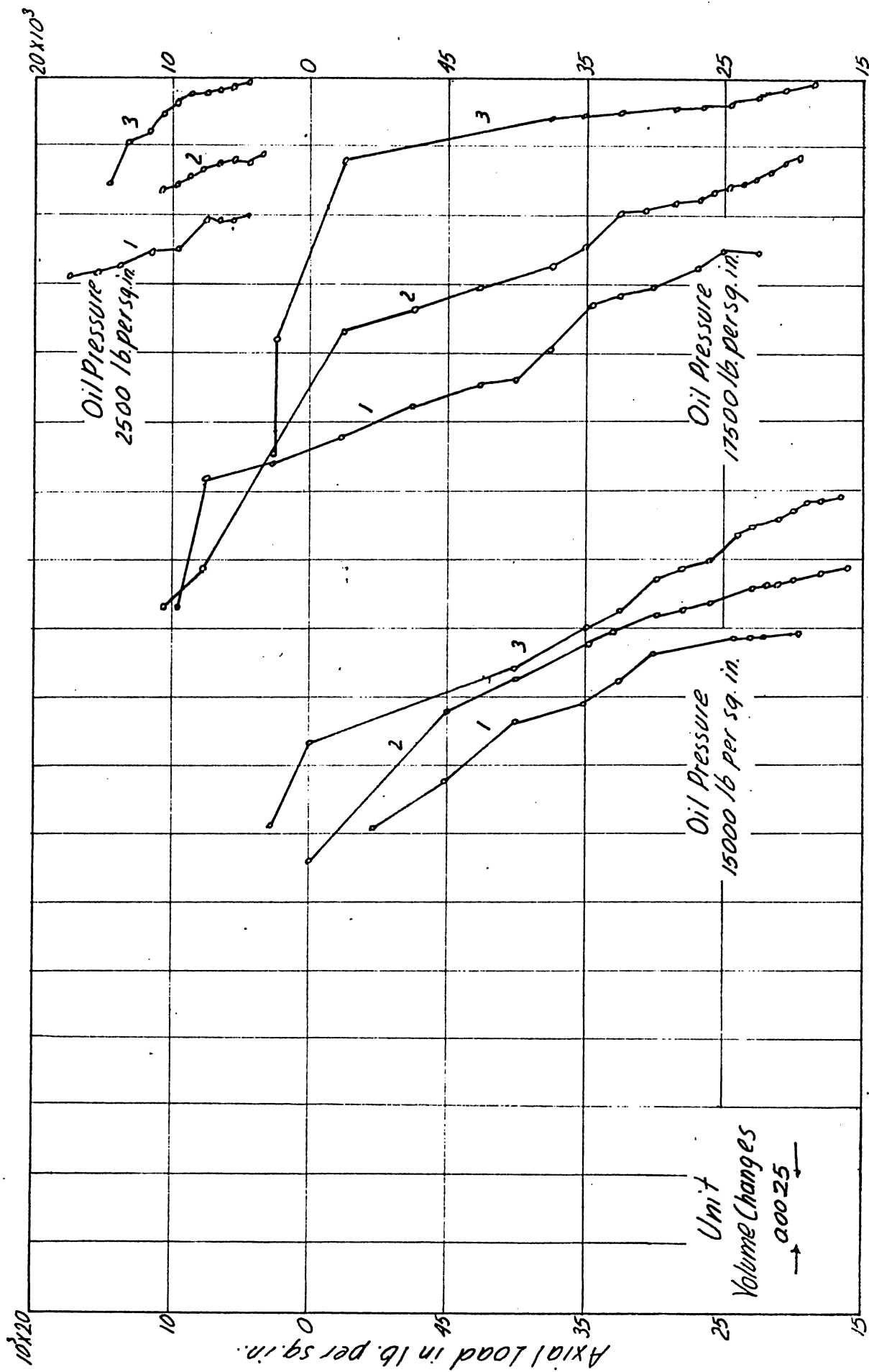


Fig 23 Volume Change Curves for Specimens with Empty Voids in Triaxial Compression
Group D



Unit Volume Change in in. cubes.

Fig 24 Volume Change Curves for Specimens with Empty Voids in Triaxial Compression
Group D



Unit Volume Change in in. cubes.
 Fig 2.5 Volume Change Curves for Specimens with Empty Voids in Triaxial Compression
 Group A

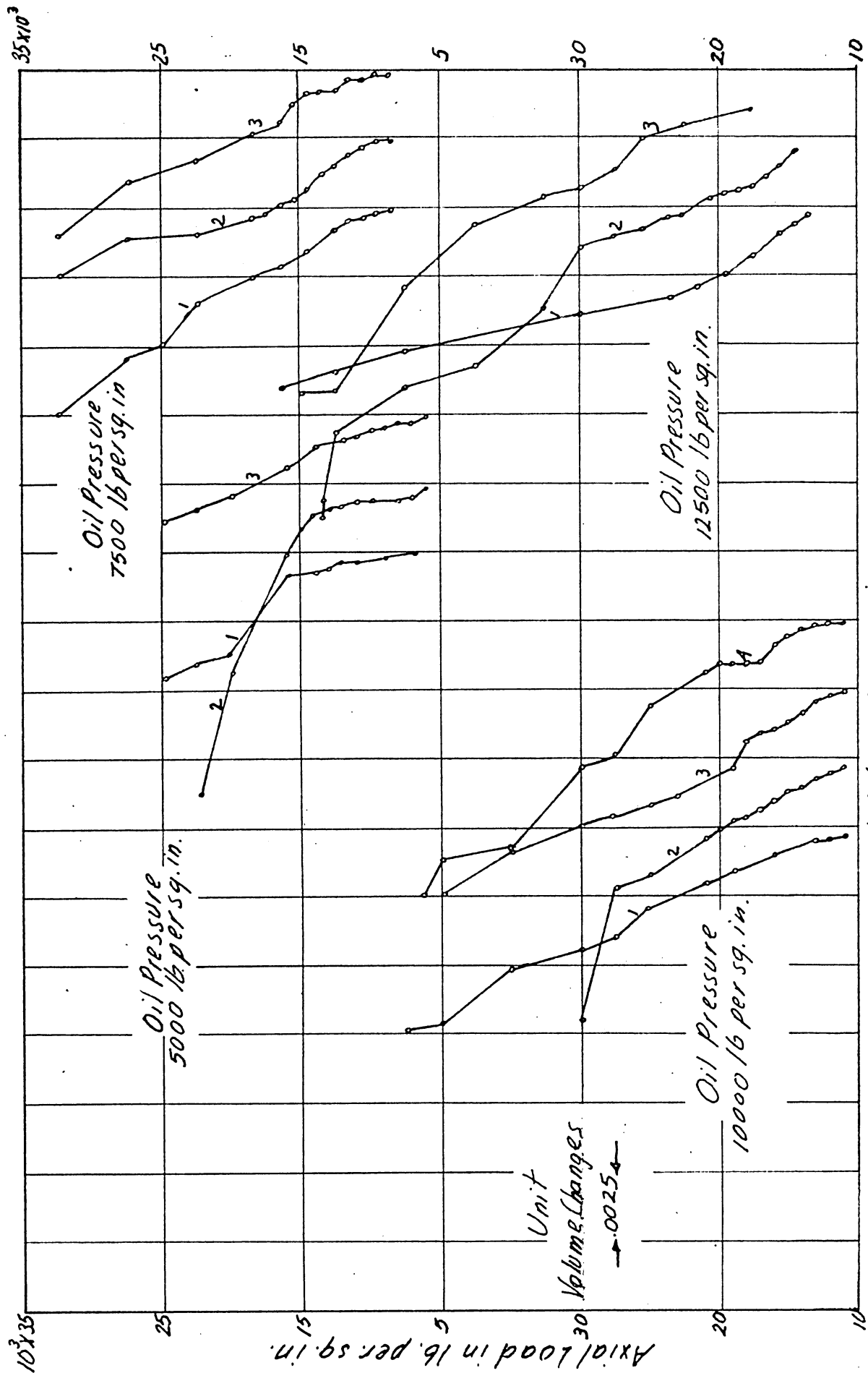
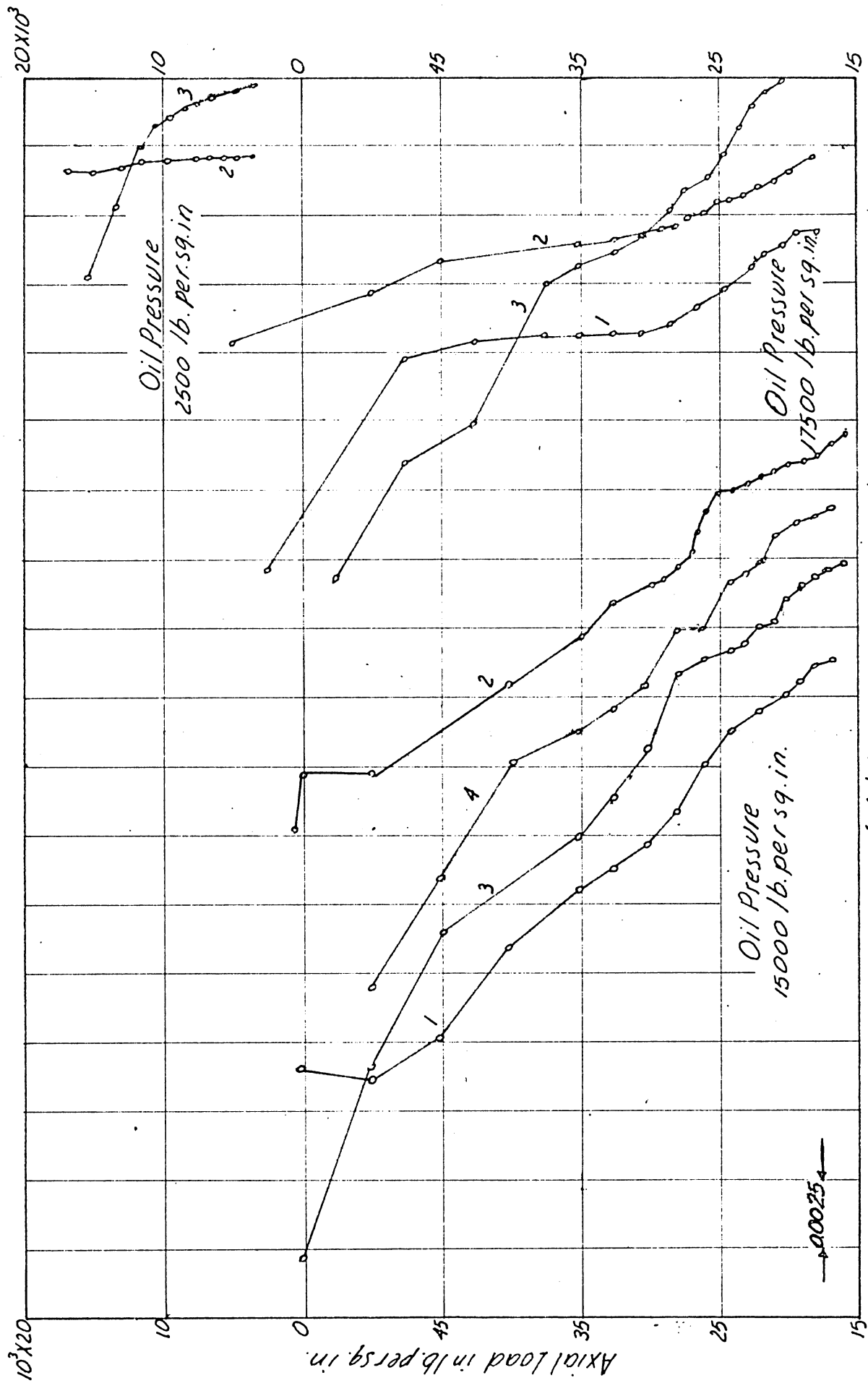
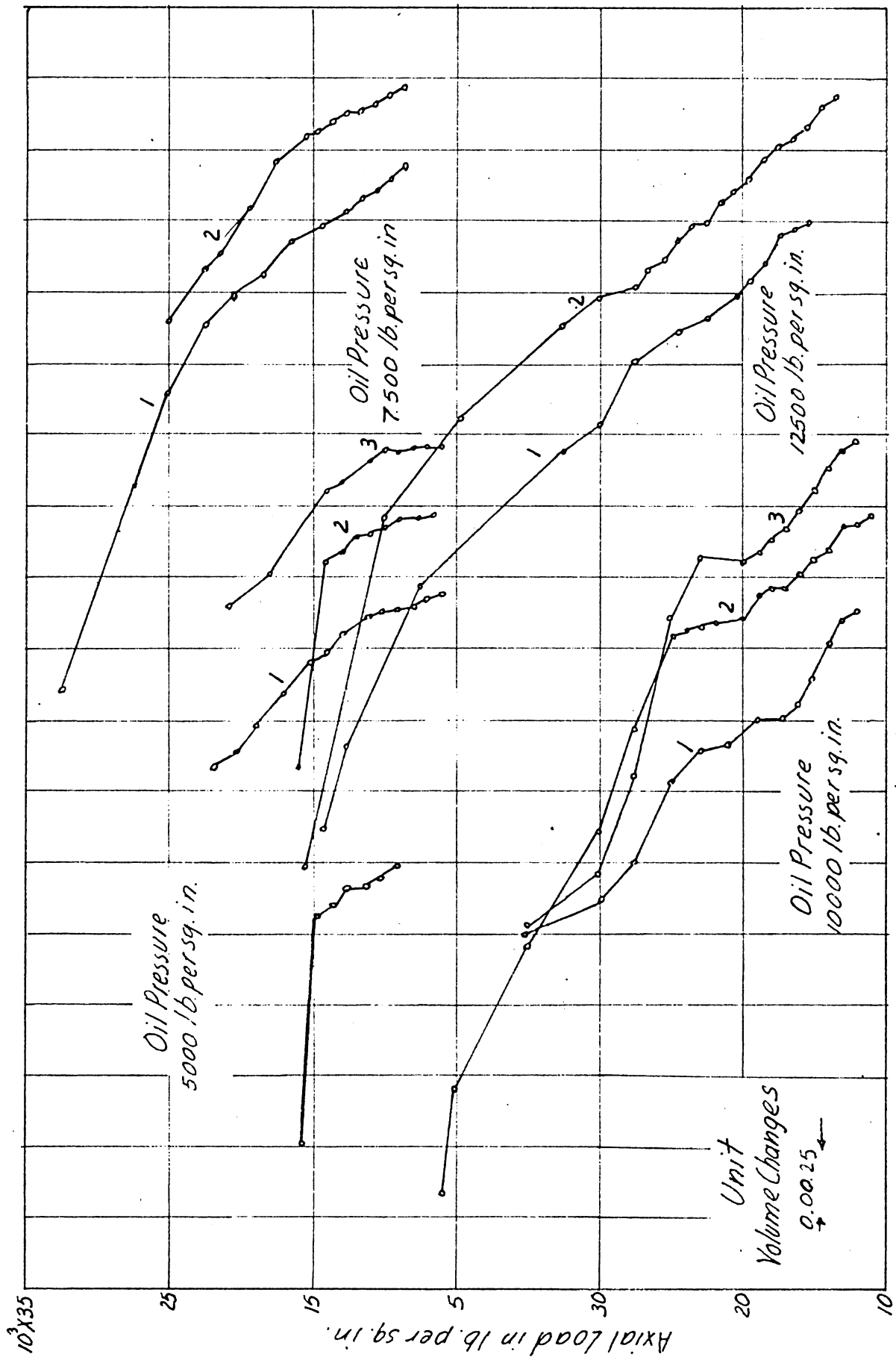


Fig 2.6 Volume Change Curves for Specimens with Empty Voids in Triaxial Compression
Group H



Unit Volume Change in in. cubes.
 Fig. 2.7 Volume Change Curves for Specimens with Empty Voids in Triaxial Compression
 Group J



Unit Volume Change in in. cubes.
 Fig 28 Volume Change Curves for Specimens with Empty Voids in Triaxial Compression
 Group J

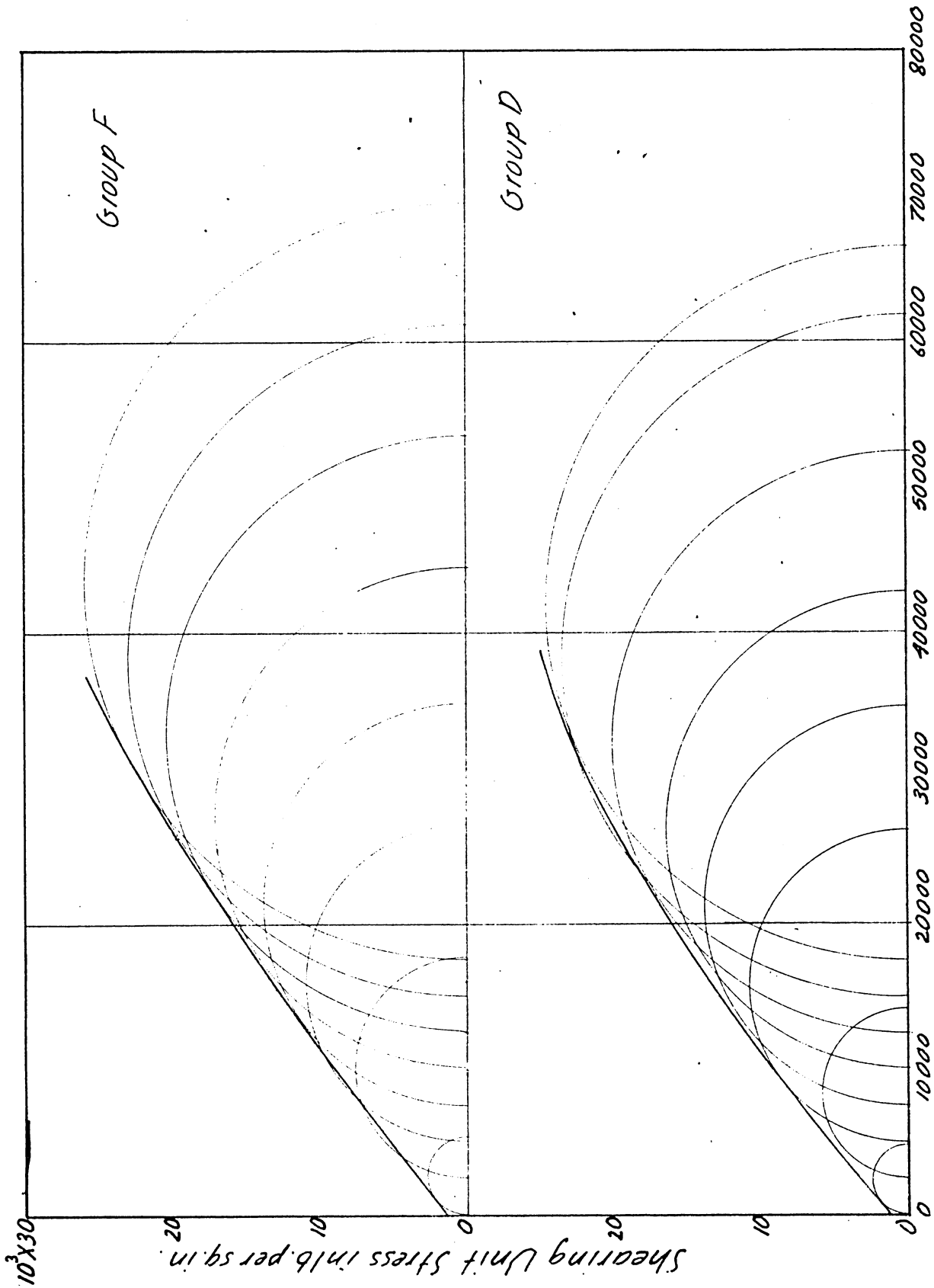


Figure 29

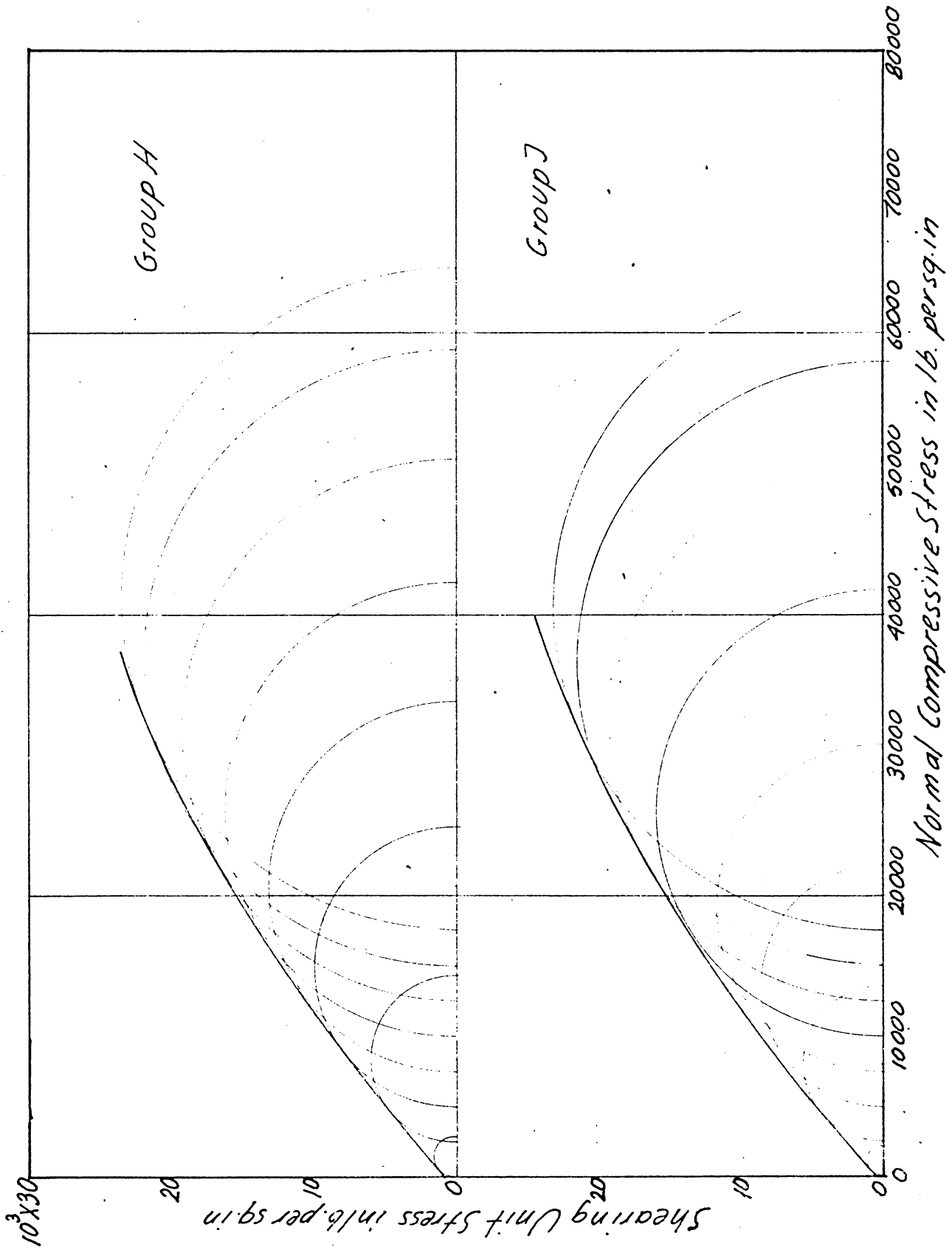


Figure 30

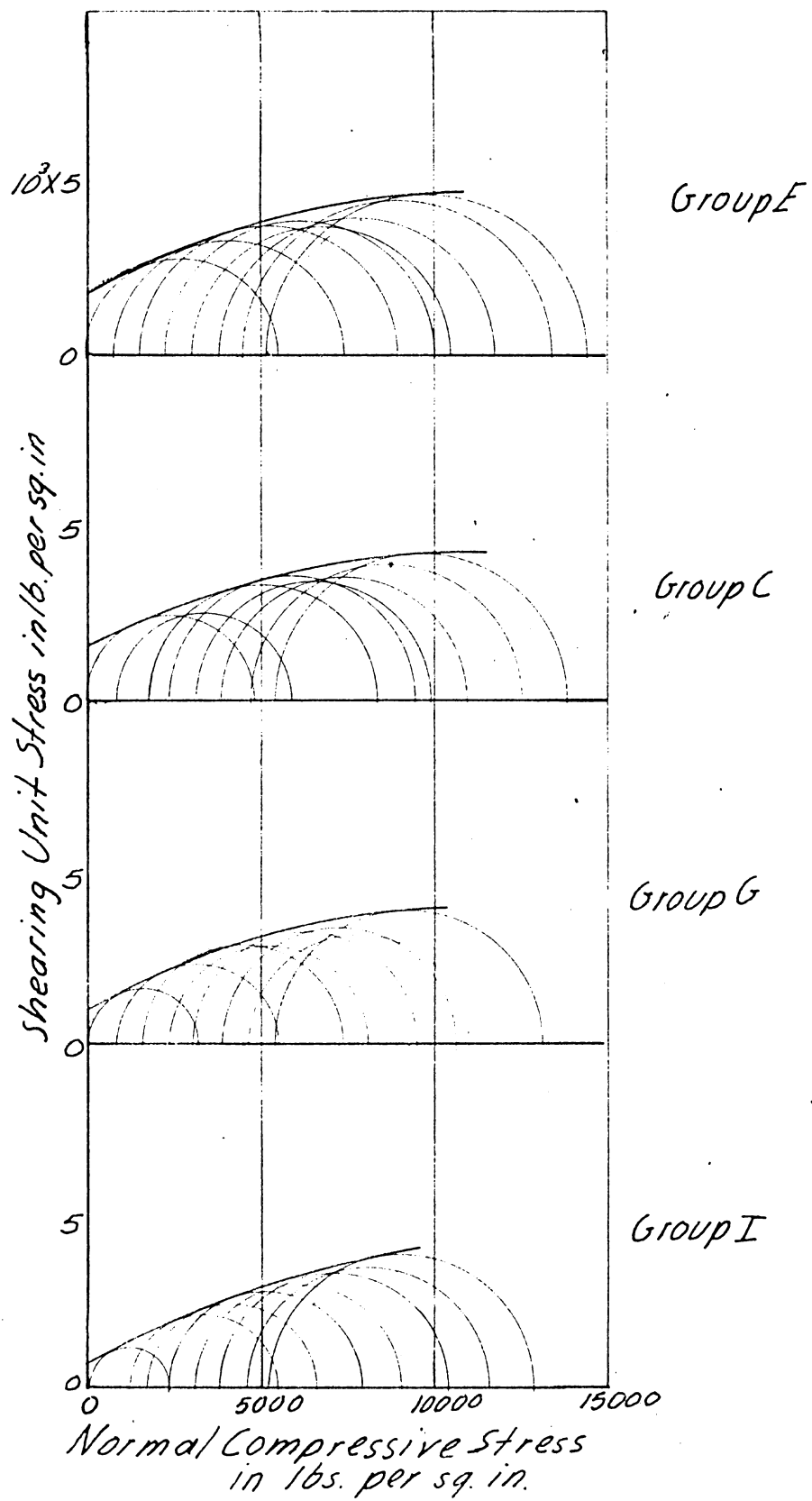
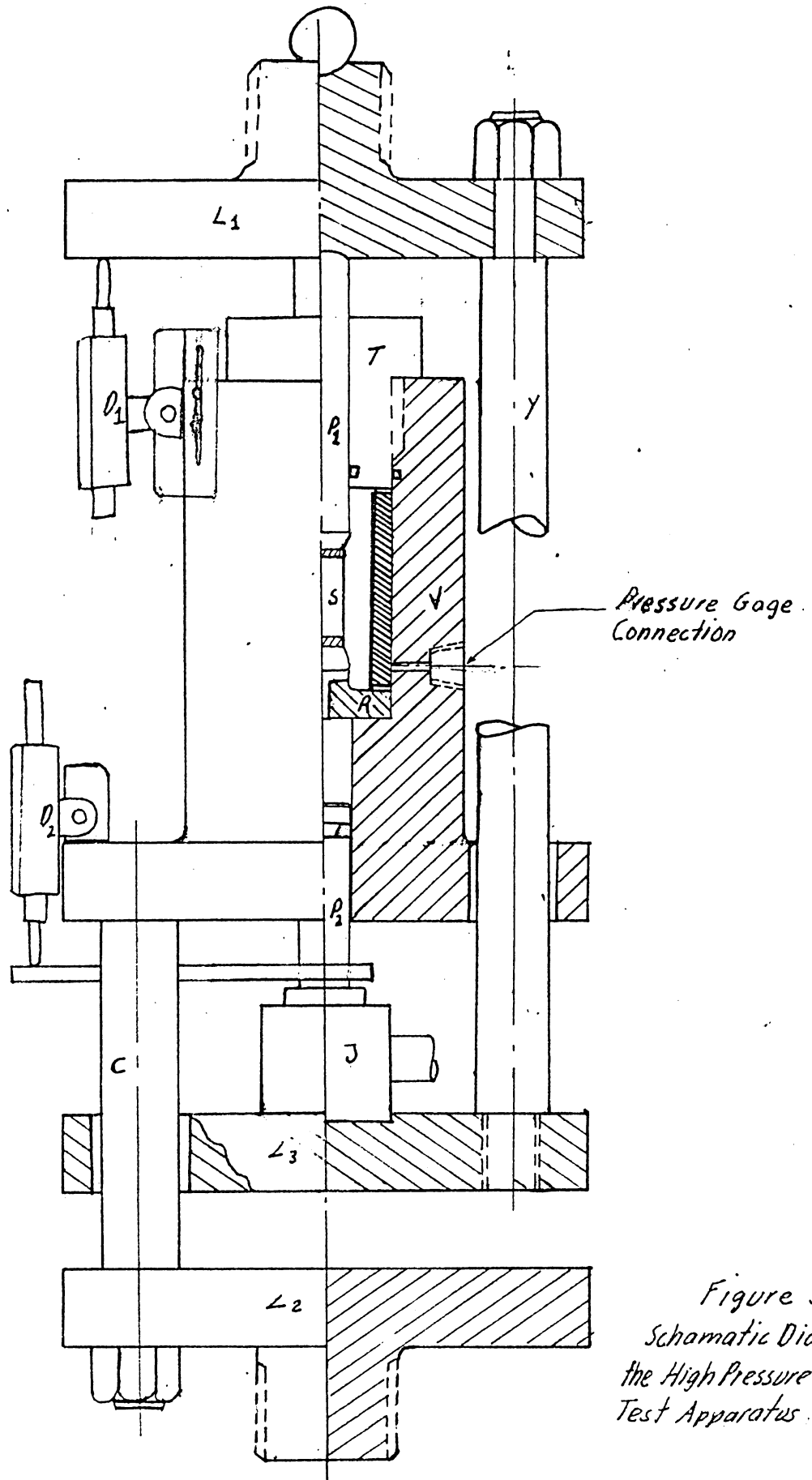


Figure 3|



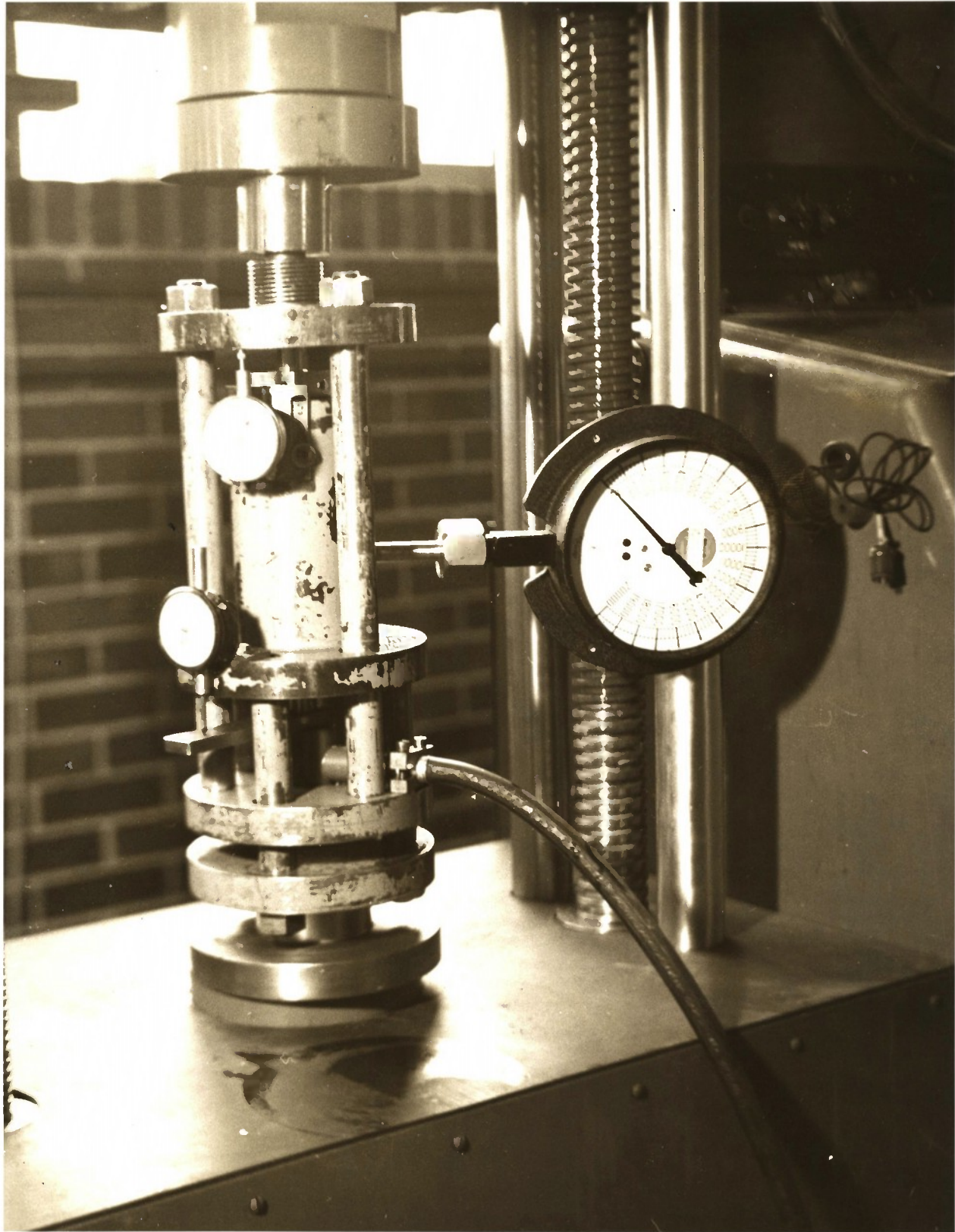


Figure 33

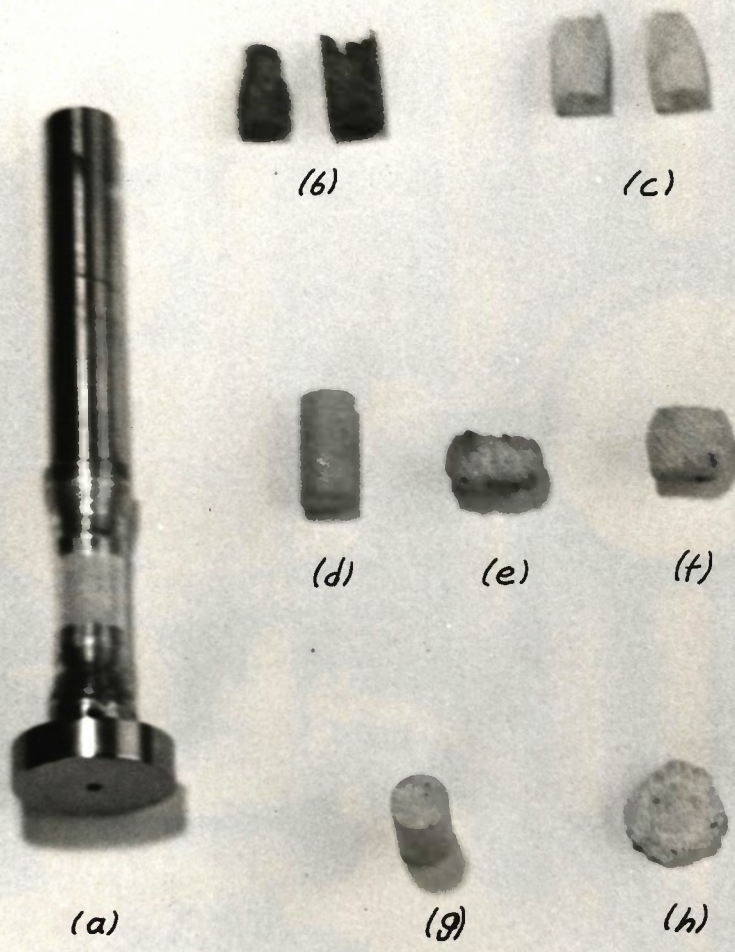


Figure 34

APPENDIX IIIRECORDED DATA AND COMPUTATIONS

In this appendix to differentiate various experiments a certain designation used, such as 2 G 2500. Where G is the concrete mixture, 2500 is the constant confining pressure, and 2 is the experiment number for that confining pressure. There are at least three experiments for each mixture at a certain confining pressure. For further information about concrete mixtures refer paragraph two and Table 1 on page 20.

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Groups		page	Concrete Mixture
	D	88	0.75:0.92:0.25
Jacketed Groups	F	105	1:2
	J	127	1:1:0.64
	H	140	1:3
	C	153	0.75:0.92:0.25
Unjacketed Groups	E	165	1:2
	G	175	1:3
	I	184	1:1:0.64

REFERENCE SHEET

Column

1	2	3	4	5	6	7	8	9	10
Confining Pressure lb/in ²	Excess Axial Load lb.				in/in 10 ⁻⁴	in/in 10 ⁻⁴	Unit Deformation in/in 10 ⁻⁴	10 ⁻⁴	Volume Change in ³ /in 10 ⁻⁴
		Lower Piston Readings			Upper Piston Readings				
		Deflection in.x10 ⁻³	Correc- tion in.x10 ⁻⁴	Corrected Deflec- tion in.x10 ⁻⁴	Deflec- tion in.x10 ⁻⁴	Correc- tion in.x10 ⁻⁴	Corrected Deflect- ion in.x10 ⁻⁴	Change in Length of Pistons Inside Chamber in x10 ⁻⁴	Volume Change Inside Chamber in. ³ x10 ⁻⁴
					Unit Deformation in/in x10 ⁻⁴	Correc- tion in in x10 ⁻⁴	Corrected Unit Deforma- tion in x10 ⁻⁴ in		
				Column 3 -Col.4			Col.6 -Col.7	Co. 8 -Co. 5	Col. 9 x 0.37 in.
		Jacketed Groups	D, F, H, J						
		Unjacketed Groups	C, E, G, I						

1D2-500

1	2	3	4	5	6	7	8	9	10
2500	0	185			145				
2500	200				44	16	28		
2500	400				65	18	47		
2500	600				79	20	59		
2500	800				90	22	68	16	6
2500	1000		52	52	100	24	72	12	4
2500	1200		8	60	109	26	83	20	7
2500	1400		3	63	12	26	92	21	8
2500	1600		8	71	131	30	101	27	10
2500	1800		3	74	145	32	113	31	11
2500	2000		8	82	162	34	128	33	14
2500	2170		13	95	216	36	180		

Ultimate load 21701b1D0

1	2	3	4	5	6	7	8	9	10
2500	0								
2500	200								
2500	400								
2500	600								
2500	800								

Ultimate loads 800 1b1D5000

1	2	3	4	5	6	7	8	9	10
2500	0	220			4				
5000	0	335			17				
5000	0	429			32				
5000	200	428.5		3	49	34	15	12	4
5000	400	428	3	6	64	36	28	22	8
5000	500	427.5	3	9	76	38	38	29	11
5000	800	426.5	8	17	89	40	49	32	12
5000	1000	425.8	5	22	101	42	59	37	14
5000	1200	425	6	28	114	44	70	42	15
5000	1400	424	8	36	129	46	83	47	17
5000	1600	422.5	13	49	151	48	103	54	20
5000	1800	420	23	72	182	50	132	60	27

Ultimate load 1800 1b

107500

1	2	3	4	5	6	7	8	9	10
0	0	0			0				
2500	0	141			15				
5000	0	230			34				
7500	0	317.5			52				
7500	200	317	3	3	60	54	6	3	1
7500	400	316.5	3	6	67	56	11	5	2
7500	600	315.5	8	14	75	58	17	3	1
7500	800	315	3	17	83	60	23	6	2
7500	1000	314	8	25	92	62	30	5	2
7500	1200	313	8	33	102	64	38	5	2
7500	1400	312	8	41	112	66	46	5	2
7500	1600	310.5	13	54	124	68	56	5	2
7500	1800	309.5	8	62	138	70	68	2	1
7500	2000	308.2	11	73	150	72	78	6	2
7500	2200	307	10	83	166	74	92	5	1
7500	2400	305.5	13	96	185	76	109	9	3
7500	2600	303.5	18	114	204	78	126	13	5
7500	2800	301.5	18	132	225	80	145	12	4
7500	3000	299.5	18	150	249	88	161	13	5
7500	3900	295	41	191	306	92	214	11	4
7500	3800	287.5	71	262	383	95	288	13	5
7500	4000	284	33	295	422	96	326	26	10
7500	4200	277.5	64	359	495	97	398	31	12
								39	14

Ultimate Load 5000 lb

10 10000

1	2	3	4	5	6	7	8	9	10
0	0	40			1				
2500	0	154			18				
5000	0	246			39				
7500	0								
10000	0	406.5			80				
10000	200	406	3	3	102	82	20	14	5
10000	400	405.5	3	6	111	84	27	18	7
10000	600	405	3	9	122	86	36	19	7
10000	800	404	8	17	133	88	45	20	7
10000	1000	403	8	25	145	90	55	22	8
10000	1200	402	8	33	151	92	69	28	10
10000	1400	401	8	41	172	94	76	27	10
10000	1600	400	8	49	188	96	88	31	11
10000	1800	399	8	57	204	98	103	38	14
10000	2000	398	8	65	222	100	122	44	16
10000	2200	396.5	13	78	242	102	140	49	18
10000	2400	396	13	91	262	104	158	41	15
10000	2600	393.5	26	117	287	106	181		
10000	2800	393			301	108	193	45	17
10000	3000	390	31	148	342	110	232	71	26
10000	3200	388.5	13	161	730	120	610	110	40
10000	5000	352.5	339	500	1130	130	1000		

Ultimate Load 6600 lb

1D12500

1	2	3	4	5	6	7	8	9	10
0	0	40			9				
2500	0	169			215				
5000	0	258			38				
7500	0	350			51				
10000	0	420			715				
12500	0	491.5			1025				
12500	500	490	10	10					
12500		490							
12500	600	490.5	4	14	116	76	30	16	6
12500	800	490.5		19	131	78	53	34	13
12500	1000	489	11	25	139	80	59	34	13
12500	1200	489		30	153	82	71	41	15
12500	1400	487.5	11	36	167	84	83	47	17
12500	1600	486.5	8	44	181	86	95	51	19
12500	1800	485.5	8	52	198	88	110	58	21
12500	2000	484	13	65	213	90	123	58	21
12500	2000	482.5	13	78	230	92	138	50	20
12500	2400	480	21	99	248	94	154	55	20
12500	2600	478.5	13	112	267	96	171	59	22
12500	2800	477	13	115	288	98	190	75	28
12500	3000				311	100	211		
12500	4000	463	140	260	500	120	380	120	44

Ultimate load 7650 lb

10 15000

1	2	3	4	5	6	7	8	9	10
2500	0	120			15				
5000	0	210			31				
7500	0	393			48				
10000	0	487			64				
12500	0	453			75				
15000	0	515			89				
15000	200								
15000	400	515			100	93	7	7	3
15000	600	515			115	95	20	20	7
15000	800	515			125	97	428	28	10
15000	1000	514.5	3	3	134	99	35	32	12
15000	1200	514	3	6	145	101	44	38	14
15000	1400	513.5	3	9	158	103	55	46	17
15000	1600	512.5	8	17	173	105	68	51	19
15000	1800	511.5	8	25	190	107	83	58	21
15000	2000	510.2	11	36	207	109	98	62	23
15000	2200	508.5	15	51	227	111	116	65	23
15000	2400	507	13	64	246	113	133	69	26
15000	2600	505.5	13	77	268	115	153	76	28
15000	2800	504	13	90	289	117	172	82	30
15000	3000	502	18	108	313	119	194	86	32
15000	3400	497.5	21	129	379	123	256	127	47
15000	4000	491.5	54	183	406	127	279	109	40
15000	5000	475	155	338	658	133	527	189	70

Ultimate load 9300 lb

2D2500 cont'd.

1	2	3	4	5	6	7	8	9	10
2500	2600	2145	23	144	212	56	156	12	4
2500					350	60	290		

Ultimate Load 3020 lb

2D0

0	0								
0	200				15	8	7		
0	400				26	10	16		
0	600				37	12	25		
0	800				48	14	34		
0	1000				58	16	42		
0	1120				67	18	49		

Ultimate load 1120 lb

2D 5000

0	0	1							
2500	0	126			14				
5000	0	217.5			37				
5000	200	217.5			40				
5000	400	216.5	8	8	61	39	10	10	4
5000	600	215.5	8	16	71	41	20	11	4
5000	800	214.5	8	24	80	43	28	12	4
5000	1000	213.5	8	32	89	45	35	11	4
5000	1400	212-220	13	45	107-109	47	42	10	4
5000	1800	219	8	53	134	53	56	11	4
5000	2200	218	8	61	158	57	77	24	9
5000	2600					61	97	36	13
5000	3000	212	52	113	225	69	156	43	16
5000	3500	206	55	168	300	73	227	59	22
5000	4000	296	95	263	400	77	323	60	22
5000	4220	257	385	648	800	80	720	72	27

Ultimate load 4220 lb

1	2	3	4	5	6	7	8	9	10
0	0	140	3	3	5	80	17	14	5
2500	0	290			25	82	23	25	9
5000	0	370			4	84	31	29	11
7500	0	453			60	86	38	33	12
10000	0	545			78	88	45	35	13
10000	200	544.5	3	3	97	90	55	39	14
10000	400	544.5		6	105	92	67	42	15
10000	600	544	3	9	115	94	78	46	17
10000	800	543.5	3	12	124	96	95	50	18
10000	1000	543	3	20	133	100	112	56	21
10000	1200	542	8	28	145	102	131	61	23
10000	1400	541	8	36	159	104	154	65	24
10000	1600	540	8	49	172	106	181	83	31
10000	1800	538.5	13	62	191				
10000	2000	537	13	75	210				
10000	2200	535.5	13	93	231				
10000	2400	533.5	18	116	256				
10000	2600	531	23	174	285				
10000	2800	525	58		353				

Oil got in
Ultimate load 2800 lb

2D-7500

0	0	45	8	8	6	55	9	9	3
2500	0	161.5	13	21	20	57	23	15	6
5000	0	256.5	13	34	36	59	34	13	5
7500	0	334	13	47	53	61	46	12	4
7500	200	334	13	60	64	63	57	10	4
7500	400	333	13	78	80	65	69	9	4
7500	600	331.5	18	90	93	67	79	1	0
7500	800	330	12	101	107	69	96	6	2
7500	1000	228.5	11	114	120	71	109	8	3
7500	1200	227	13	127	134	73	127	13	5
7500	1400	225.7	13	145	146	75	146	19	7
7500	1600	224.3	18	168	165	78	164	19	7
7500	1800	223	23	191	180	79	191	23	8
7500	2000	221.5	23		200	81	212	21	8
7500	2200	220	18		221				
7500	2400	218	23		241				
7500	2600	215.5	23		270				
7500	2800	213	23		293				

Cont'd. on next page.

2D-7500Cont'd.

1	2	3	4	5	6	7	8	9	10
7500	3000	210.5	23	214	325	83	242	28	10
7500	3600	200	99	313	455	86	369	56	21
7500	4000	190	94	407	575	88	487	80	30
7500	4400								

Ultimate Load 6000 lb.

2D 12500

		25			3				
2500	0	155			19				
5000	0	247			315				
7500	0	328			465				
10000	0	405			64				
12500	0	484			85				
12500	200	484			92	87	5	5	2
12500	400	484			102	89	13	13	5
12500	600	483	6		111	91	20	14	5
12500	800	482.5	3		120	93	27	18	7
12500	1000	482	3		131	95	36	24	9
12500	1200	481.5-487.5	3		142	97	45	30	11
12500	1400	488	5		156-160	103	57	37	14
12500	1600	488	6		179	105	74	48	18
12500	1800	486.5	11		220	107	113	76	28
12500	2000	485.5	8		236	109	127	82	30
12500	2200	484.5	8		250	111	139	86	32
12500	2400	483	13		270	113	157	81	30
12500	2600	481	18		300	115	185	101	37
12500	2800	478.5	23		328	117	211	104	39
12500	3000	476.5	18		350	119	231	106	40
12500	3500	468.5	75		448	123	317	117	43
12500	4000	460	75		525	127	398	123	45
12500	6000	426	270		990	141	849	254	94
12500	7000	405	200		1300	147	1153	358	132

Ultimate Load 8400 lb

2D 15000

1	2	3	4	5	6	7	8	9	10
2500	0	40			25				
5000	0	140			41				
7500	0	231			59				
10000	0	316			82				
12500	0	397			112				
15000	0	475			147				
15000	0	540			185	149	36	36	13
15000	200	543.5							
15000	400				202	153	49	49	18
15000	600				214	155	59	59	22
15000	800				228	157	71	67	25
15000	1000	543	4	4	248	159	89	76	28
15000	1200	542	9	13	264	161	103	81	30
15000	1400	541	9	22	282	163	119	74	27
15000	1600	539.5	13	35	305	165	140	97	36
15000	1800	538	8	43	331	167	164	109	40
15000	2000	536.5	13	56	353	169	184	115	43
15000	2200	535	13	69	378	171	207	125	46
15000	2400	533	13	82	400-409	182	227	132	49
15000	2600	531-536	13	95	450	184	266	158	58
15000	2800	536	13	108	475	186	289	168	62
15000	3000	534.5	13	121	554	190	364	183	68
15000	3500	528	60	181	542	194	448	172	64
15000	4000	518	95	276	848	209	639	183	68
15000	5000	507	85	456	1165	215	950	184	68
15000	6000	575	310	766					

Ultimate load 9000 1b

2D 17500

1	2	3	4	5	6	7	8	9	10
2500	0	108			14				
5000	0	194			335				
7500	0	275			525				
10000	0								
12500	0	224			10				
15000	0	494			135				
17500	0	560			180				
17500	200	560			186	182	4	4	1
17500	400	560			194	184	10	10	4
17500	600	559		8	206	186	20	12	4
17500	800	557.5		21	220	188	31	10	4
17500	1000	556	13	34	236	190	46	12	4
17500	1200	554	18	52	253	192	61	9	3
17500	1400	552.5	13	65	270	194	76	11	4
17500	1600	551	13	78	287	196	91	13	5
17500	1800	548.5	23	101	31	198	111	10	4
17500	2200	544	41	139	356	202	148	9	3
17500	2400	542				204			
17500	2600	540	36	145	404	206	198	53	20
17500	2800					208			
17500	3000	535	46	191	454	210	244	53	20
17500	4000	520	140	331	600	218	382	51	19
17500	5000	505	140	471	855	226	529	48	28
17500	6000	488.5	165	636	1050	232	800	164	61
17500	7000	462	255	891	1300	238	1062	271	100

Ultimate load 9600 lb

				300	
0	200			10	2
0	400			14	4
0	600			18	6
0	800			23	9
0	1000			29	13
				8	
				10	
				12	
				14	
				16	

Ultimate Load 1000 lb

	1	2	3	4	5	<u>3D 2500</u>		7	8	9	10
2500	0	0	55			01					
2500	0	0	177.5			16					
2500	200	200	6.5	8	8	41		18	23	15	6
2500	400	400	5.3	10	18	62		20	42	24	9
2500	600	600	4.1	11	29	75		22	53	24	9
2500	800	800	3.5	4	33	85		24	61	28	10
2500	1000	1000	3	3	36	96		26	70	34	12
2500	1200	1200	2.2	6	42	103		28	75	33	12
2500	1400	1400	1.8	2	44	119		30	89	45	17
2500	1600	1600	1	6	50	130		32	98	48	18
2500	1800	1800	0	8	58	146		34	112	54	20
2500	1840					234		34	200		

1840 lb Failure

	<u>3D-5000</u>		7	8	9	10
2500	0	157				
5000	0	262				
5000	0					
5000	200	274				
5000	400	273.5	3	28	20	7
5000	600	273	3	30	31	9
5000	800	272.5	3	32	41	13
5000	1000	272.5	3	34	51	15
5000	1200	271.5	3	36	59	17
5000				38	67	19
5000				40	76	
5000	1800	269	19	44	83	18
5000	2000	266.5	23	46	111	20

Oil got in Ultimate load 2000 lb

3-D-7500

1	2	3	4	5	6	7	8	9	10
2500	0	196			21	58	6		
5000	0	283			38	60	19	16	
7500	0	360.5			56	62	28	17	6
7500	200	360.5			64	64	36	17	6
7500	400	360	3	3	79	66	45	18	6
7500	600	359	8	11	90	68	53	18	7
7500	800	358	8	27	100	70	62	19	7
7500	1000	357	8	35	111	72	71	20	7
7500	1200	356	8	43	121	74	82	23	7
7500	1400	355	8	51	132	76	93	21	8
7500	1600	854	8	59	143	78	106	21	8
7500	1800	353	8	72	156	80	120	22	8
7500	2000	351.5	13	85	169	82	136	25	9
7500	2200	350	13	98	184	84	154	25	9
7500	2400	348.5	13	111	200	86	174	27	10
7500	2600	347	13	129	218	94	334	37	14
7500	2800	345	18	147	238	98	502	65	24
7500	3000	343	18	297	260				
7500	4000	327	150	437	438				
7500	4500	312	140		600				

Ultimate load 5200 lb

3D-10000

1	2	3	4	5	6	7	8	9	10
0									
2500		50			19				
5000		178			385				
7500		272			58				
10000		354			81				
10000		430			90				
10000	200	430		8	99		7		
10000	400	429		16	101	83	14	7	3
10000	600	428	8	24	120	85	23	6	2
10000	800	427	8	32	132	27	31	7	3
10000	1000	426	8	40	143	89	42	7	3
10000	1200	425	8	48	155	91	50	10	4
10000	1400	424	8	56	169	93	60	10	4
10000	1600	423	8	68	182	95	72	12	4
10000	1800	421.6	12	82	198	97	83	16	6
10000	2000	420	14	90	213	99	97	15	6
10000	2200	419	8	103	230	101	110	15	6
10000	2400	417.5	13	116	258	103	125	20	20
10000	2600	416	13	134	270	105	146	22	8
10000	2800	414	18	152	294	107	161	30	11
10000	3000	412	18	252	450	109	183	27	10
10000	9000	401	100	432	670	111	331	31	11
10000	5000	382	180	792	1050	119	543	79	29
10000	6000	345	360			127	917	111	41
						133		125	46

Ultimate load 6400-1b

1	2	3	4	5	6	7	8	9	10
2500	0	8			76				
5000	0	167			28				
	0								
12500	0	401.5	3		104		14	11	4
12500	200	401	8		121		29	18	7
12500	400	400	13	11	137		46	22	8
12500	600	398.5	13	24	156		66	29	10
12500	800	397	13	37	178		80	30	11
12500	1000	395.5	13	50	194		105	44	16
12500	1400	393	11	61	223		130	58	21
12500	1800	391.5	11	72	252		176	83	30
12500	2200	389-398.5	21	93	288-292		205	96	35
12500	2600	396.5	16	109	335		240	125	46
12500	3000	394.5	16	115	374		289	144	53
12500	3500	391	30	145	427		340	145	53
12500	4000	386.5	50	195	482		465	165	61
12500	5000	375	105	300	615		624	194	72
12500	6000	361	130	430	780		880	320	119
12500	7000	347	130	560	1040		1780	305	113
12500	7620	265	815	1375	1946				

Ultimate load 7620 lb

3D 15000

0	0	0	0	0	0	0	0	0	0
2500	0	140			12				
5000	0	220			25				
7500	0				1				
10000	0	370			46				
12500	0	465			57				
15000	0	534.5			68				
15000	0	534.5			77				
15000	200	533.0			90	70	7		
15000	400	533			101	72	18		
15000	600	532.5			111	74	27		
15000	800	532			121	76	35		
15000	1000	531			13	78	44		
15000	1200	530.2			139	80	50		
15000	1400	529.5			148	82	57		
15000	1600	528.4			15	84	64		
15000	1800	527			176	86	74		
15000	2000	523			210	88	88		
15000	2200					90	120		

Oil got in Ultimate
Load 2200 lb

<u>4D 17500</u>									
1	2	3	4	5	6	7	8	9	10
2500	0	90			14				
5000	0	179			26				
7500	0	258			42				
10000	0	335			63				
12500	0	406			86				
15000	0	464			111				
17500	0	536.5			1485				
17500	200	536	3	3	156	150	6	3	1
17500	400	535.5	3	6	162	152	10	4	1
17500	600	535	3	9	168	154	14	5	2
17500	800	534.5	3	12					
17500	1000	533.8	5	17	189	158	31	14	5
17500	1200	533	6	23	200	160	40	17	6
17500	1400	532	8	31	213	162	51	20	7
17500	1800	530	16	47	242	166	76	29	10
17500	2000	529	8	55	255	168	87	32	12
	2400	526	21	76	288	172	106	30	11
17500	2600	525	13	89	304	180	124	35	13
17500	4000	523.5	45	134	443	188	255	121	45
17500	5000	517.5	50	184	538	196	342	158	59
17500	6000	508.2	82	266	648	202	446	180	67
17500	7000	497.5	97	363	770	212	558	195	72
17500	8000	485	115	478	918	218	700	222	82
17500	9000	472				224	884		
17500	9900	416			1108				

Ultimate load 9900 lb

500

3
7
11
15

Ultimate load 780 lb

0
0
0
0

	<u>IFO</u>									
	1	2	3	4	5	6	7	8	9	10
10		0				10	8	2		
		200				14.5	10	4.5		
		400				19	12	7		
		600				24	14	10		
		800				32	16	16		
		1000								

Ultimate 1000 lb.

	<u>IF2,500</u>									
	1	2	3	4	5	6	7	8	9	10
2500		0	113	-	-	22	24	12	12	4
	200	200	113	-	-	36	26	29	21	8
	400	400	112	8	8	55	28	42	26	10
	600	600	111	8	16	70	30	55	31	11
	800	800	110	8	24	85	32	63	31	11
	1000	1000	109	8	32	95	34	73	33	12
	1200	1200	108	8	40	107	36	82	34	13
	1400	1400	107	8	48	118	38	94	38	14
	1600	1600	106	8	56	132	40	105	36	13
	1800	1800	104.5	13	69	145	42	120	33	12
	2000	2000	102.5	18	87	162	44	140	30	11
	2200	2200	100	23	110	184	46	164	31	12
	2400	2400	97.5	25	133	210	48	202	36	13
	2600	2600	94	33	166	250	50	277	88	33
	2800	2800	91.5	23	189	327				

Ultimate 288016

Oct 30-6
IF 5000

1	2	3	4	5	6	7	8	9	10
0	1	160	8	8	23	41	8	0	0
2500	200	2595	8	16	395	43	17	1	0
5000	4	2585	8	24	49	45	24	0	0
	6	2575	8	32	60	47	32	0	0
	8	2565	8	35	69	49	37	2	1
	1000	2555	3	43	79	51	44	1	0
	12	255	8	51	86	53	50	0	0
	14	254	8	59	95	55	59	0	0
	16	253	8	67	103	57	68	1	0
	18	252	8	75	114	59	80	5	2
	2000	2505	8	88	125	61	91	3	1
	22	249	13	106	139	63	106	0	0
	24	2475	18	124	152	65	122	0	0
	26	2455	18	142	169	67	141	0	0
	28	2435	18	165	187	69	169	4	1
	3000	2415	23	225	208	73	230	5	2
	3500	239	60	345	228	77	358	13	5
	4000	2325	120		305				
		220			435				

Ultimate 4300 1b

Oct 30-1
IF 10000

1	2	3	4	5	6	7	8	9	10
0	0								
2500		250		1.5	13				
5000		315		1.8	36				
7500		389		3.3	485				
10000		473		5.4	60				
		470		7.1	62	62			
	200	469	6	6	73	64	9	3	1
	4	468	8	14	82	66	17	3	1
	6	467	8	22	90	68	22	0	
	8	466	8	30	98	70	28	-2	1
1000	1000	465	3	33	105	72	33	0	
12	12	464	3	36	113	74	39	3	1
14	14	463	8	44	124	76	48	4	1
16	16	461.5	8	52	136	78	58	6	1
1800	1800	460	13	65	147	80	67	2	2
2000	2000	458.5	13	78	16	82	78	0	1
22	22	457	13	91	178	84	94	3	
24	24	452.5	13	104	195	86	109	52	15
2600	2600	443	41	145	275	90	185	40	39
3000	3000	429	85	150	35	94	256	106	45
4000	4000	410	130	280	498	98	400	120	
5000	5000		180	360	695	101	584	234	81

Ultimate 64001b

<u>Oct 30-4</u>		<u>IF 15000</u>									
1	2	3	4	5	6	7	8	9	10		
2500	0	10			2						
5000	0	125			17						
7500		215			30						
10000		300			42						
12500		378			58						
15000		457			80						
		527			107						
	200	527	0		116	110	6				
	400	527	0		127	112	15				
	600	526.8	0		136	114	22				
	800	526.5	1	1	145	116	30	29			
	1000	526	3	4	154	118	36	32			
	1200	525	8	12	162	120	42	30			
	1400	524	8	20	173	122	51	31			
	1600	523	8	28	184	124	60	32			
	1800	522	8	36	197	126	71	35			
	2000	521	8	44	211	128	83	39			
	2200	520	8	52	224	130	84	42			
	2400	518.8	10	62	237	132	105	43			
	2600	517.5	11	73	252	134	118	45			
	2800	515.8	15	88	269	136	123	35			
	3000	514	16	104	288	138	150	46			
	3500	510.5	30	134	333	142	191	57			
	4000	507	30	164	380	146	234	70			
	4500	503.5	30	194	433	150	283	89			
	5500	93.5	85	279	546	154	392	123			
	6000	488.5	45	323	608	157	451	128			
	7000	74.5	130	453	759	160	599	146			
	8000	52.5	210	663	942	163	790	127			
	9000	23.5	280	943	1390	166	1230	287			
										11	11
										12	12
										11	11
										11	11
										12	12
										13	13
										14	14
										15	15
										16	16
										17	17
										13	13
										17	17
										21	21
										26	26
										33	33
										45	45
										47	47
										54	54
										47	47
										106	106
										<u>Ultimate 92801b</u>	

1	2	3	4	5	6	7	8	9	10
2500		127.5	0	0	18	20	8	3	3
	200	127.5	3	3	28	22	16	13	5
	400	127	6	9	38	24	24	15	6
	600	126	8	17	48	26	31	14	5
	800	125	8	25	58	28	39	14	5
	1000	124	8	32	67	30	46	14	5
	1200	123	8	40	76	32	55	15	5
	1400	122-131	8	48	87	34	66	18	6
	1600	130	8	56	160	36	77	21	7
	1800	119	8	64	113	38	90	26	8
	2000	118	8	77	128	40	110	33	10
	2200	116.5	13	90	150	42	129	31	12
	2400	115	13	118	163	44	154	36	14
	2600	112	28	156	198	46	196	40	15
	2800	108	38		242				
	3000								

Ultimate 29801b2F 7,500

2500	196	361	360.5	357	356	355	354	353	352	351	349.5	348	346.5	345	339	332	5000
5000	271	361	360.5	357	356	355	354	353	352	351	349.5	348	346.5	345	339	332	5000
7500	361	361	360.5	357	356	355	354	353	352	351	349.5	348	346.5	345	339	332	5000
	200	200	200	200	200	200	200	200	200	200	200	200	200	200	200	200	200
	400	400	400	400	400	400	400	400	400	400	400	400	400	400	400	400	400
	600	600	600	600	600	600	600	600	600	600	600	600	600	600	600	600	600
	800	800	800	800	800	800	800	800	800	800	800	800	800	800	800	800	800
	1000	1000	1000	1000	1000	1000	1000	1000	1000	1000	1000	1000	1000	1000	1000	1000	1000
	1200	1200	1200	1200	1200	1200	1200	1200	1200	1200	1200	1200	1200	1200	1200	1200	1200
	1400	1400	1400	1400	1400	1400	1400	1400	1400	1400	1400	1400	1400	1400	1400	1400	1400
	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000
	2200	2200	2200	2200	2200	2200	2200	2200	2200	2200	2200	2200	2200	2200	2200	2200	2200
	2400	2400	2400	2400	2400	2400	2400	2400	2400	2400	2400	2400	2400	2400	2400	2400	2400
	2600	2600	2600	2600	2600	2600	2600	2600	2600	2600	2600	2600	2600	2600	2600	2600	2600
	2800	2800	2800	2800	2800	2800	2800	2800	2800	2800	2800	2800	2800	2800	2800	2800	2800
	3000	3000	3000	3000	3000	3000	3000	3000	3000	3000	3000	3000	3000	3000	3000	3000	3000

Ultimates 4201b

<u>2F 10000</u>										
1	2	3	4	5	6	7	8	9	10	
2500		161			28					
5000		252			48					
10000		411.5			103					
	200	411	3	3		105				
	400	410	8	11	133	107				
	600	408.5	13	24	149	109				
	800	407.5	8	32	163	111				
	1000	406.5-412	8	40	177-180	116				
	1400	411	6	46	208	120				
	1800	409.3	13	59	240	124				
	2200	407	19	78	273	128				
	2600	404	26	104	311	132				
	3000	400.5	31	135	353	136				
	3500	395	50	185	412	140				
	4000	389	55	240	480	144				
	5000	378-384	110	350	650	148				
	6000	370	130	480	875	152				

Ultimate 6240 1b

1	2	3	4	5	Nov. 5-1		7	8	9	10
					2F	12,500				
2500		263			18					
5000		255			33					
7500		341			50					
10000		423			68					
12500		507			93					
	200	507			102		3	3	3	1
	400	506.5	3	3	109		12	9	9	3
	600	506	3	6	119		20	14	14	5
	800	505.5	3	9	130		29	20	20	7
	900	504.5	8	17	140		37	20	20	7
	1000	503.5	8	25	151		46	21	21	8
	1200	502.5	8	32	162		55	23	23	8
	1400	502	3	35	175		66	31	31	11
	1600	501	8	43	186		75	32	32	12
	1800	500	8	51	198		85	34	34	13
	2000									
	2200	499	6	57	212		75	38	38	14
	2400	498	8	65	225		106	41	41	15
	2600	497	8	73	238		117	44	44	16
	2800	495.5	13	86	252		129	42	42	16
	3000	494	13	99	266		141	42	42	16
	4000	487.5	55	154	350		217	57	57	21
	5000	479	75	229	455		314	85	85	32
	6000	69	90	319	578		431	112	112	42
	7000	55	90	409	733		570	161	161	50
	8000	30	240	649	101.1		842	193	193	71

Ultimate 8220 1b

Nov. 4-1
2F 17,500

1	2	3	4	5	6	7	8	9	10
2500		126			21	172	5		1
5000		218			38	173	10	2	1
7500		300			60	175	15	4	1
10000		375			81	177	26	6	2
12500		455			111	179	36	9	3
15000		522.5			138	181	46	11	4
17500		574.5			176	183	59	14	4
	200	574.5			172	185	72	14	4
	400	574	31	3	178	187	86	10	4
	600	573.5	3	6	185	189	101	12	4
	800	573	3	9	192	191	117	15	5
	1000	572	8	17	205	193	133	23	9
	1200	571	8	25	217	195	151	29	10
	1400	570	8	32	229	197	167	31	11
	1600	568.5	13	45	244	199	260	34	13
	1800	567	13	58	259	207	368	52	19
	2000	565	18	76	275	215	475	79	29
	2200	563.5	13	89	292	221	535	100	37
	2400	562	13	102	310	227	617	161	57
	2600	560	18	110	328	233	928		
	2800	558.5	13	123	348	240			
	3000	557	13	136	366				
	4000	547	90	226	467				
	5000	537	90	316	583				
	6000	526	80	396	696				
	7000	520	30	426	762				
	8000	526	30	456	850				
	9000				1168				

Ultimate 9400 lb

0

0
200
400
600
800
1000

11 8 3
17 10 7
25 12 13
34 14 20
44 16 36

Ultimate 1100 lb

3F 2500

2500

154.5
154.5
154
153
152
151
149
147
144
140
129.5
94
10

3
8
8
8
16
16
26
36
101
355

3
11
19
27
43
59
85
121
222
577

10
38
45
54
64
83
107
136
180
287
700
1500

12 24
14 29
16 36
18 44
20 61
22 83
24 110
26 152
28 257
30 662
32

21
18
17
17
18
24
25
31
35
85

8
7
6
6
7
9
9
11
13
31

Ultimate 3060 lb

Jan 2-2 3F 5000

2500
5000

140
226.5
226.5
226
224.5
222.5
221
216.5
215-220.5
219
216
212.5
206
194
160
80

0
5
15
20
15
39
15
15
30
35
65
120
444

21
41
54
71
86
111
130
171
185-187
224
264
304
384
505
2000
2800

0
5
20
40
55
94
109
114
144
179
244
364
808

43
45
47
49
51
53
55
57
59
61
65
69
73

11
26
39
62
79
118
132
167
205
243
319
436
1875

11
21
19
22
24
24
23
43
61
64
75
72
1067

4
8
7
8
9
9
9
16
23
24
28
27
370

Ultimate 4400 lb

[illegible]

Nov. 10-1
3F 12,500

1	2	3	4	5	6	7	8	9	10
2500		100							
5000		222							
7500		301							
10000		384							
12500		462							
		538			113				
		538		8	118	115	3	3	1
		537		16	1295	117	12	4	1
		536		24	137	119	18	2	1
		535		27	146	121	25	1	1
		534.5		30	155	123	32	5	0
		534		38	166	125	41		2
		533		51	177	127	50		4
		531.5		64	191	129	62		4
		530		77	206	131	75		4
		528.5		85	221	133	88		4
		527.5		98	240	135	105		4
		526		112	257	139	122	20	7
		524.5		125	275	143	132	24	9
		523		138	295	147	148	20	7
		521.5		173	318	151	167	23	9
		517.5		213	380	155	225	29	10
		513		258	441	159	282	52	19
		508		313	550	163	387	69	25
		502		453	678	167	511	129	48
		487		673	84.3	173	670	138	50
		466		973	107.6	179	897	169	63
		435			1500	182	1418	224	84
								445	105

Ultimate 8600 1p

4F 12500

1	2	3	4	5	6	7	8	9	10
0	0				5				
2500		100			19				
5000		208			34				
7500		298			50				
10000		382			68				
12000		467			105				
		539			109				
	200	539			118		5	5	2
	400	538.5	3	3	118	106	12	9	3
	600	537.5	8	11	130	108	22		4
	800	536.5	8	19	142	110	32	11	8
	1000	534	13	32	157	112	45	23	5
	1200	532.5	13	45	171	114	57	13	5
	1400	531.5-530	8	53	184	116	68	12	5
	1600	529	8	61	197	118	79	15	6
	1800	527	16	77	215	120	95	18	7
	2000	525	16	93	231	122	109	18	7
	2200	523.5	13	106	248	124	124	16	6
	2400	521.5	16	122	266	126	140	18	7
	2600	519	16	145	288	126	160	15	6
	2800	517	23	163	308	130	178	15	6
	3000	515	18	181	327	132	197	16	6
	4000	502	120	301	470	136	334	33	12
	5000	485	160	461	666	140	520	59	22
	6000	453.5	305	766	984	143	841	65	24
	7000	395	585	1351	1590	146	1444	93	34

Ultimate 7380 1p

4F 15000

1	2	3	4	5	6	7	8	9	10
25		77			31				
50		157			45				
75									
10		297			85				
12		31.5			110				
15		1	3	3	118	112		3	1
	200	0.5	3	6	125	114		5	2
	400	70	3	9	135	116		10	4
	600		8	17	146	118		11	4
	800	169	8	25	159	120		14	5
	1000	168	8	32	172	122		18	7
	1200	167	8	40	184	126		18	7
	1400	166	8	48	196	128		20	7
	1600	165	8	56	208	130		22	8
	1800	164	8	67	238-242	136	108	41	15
	2200	162.5	11	78	279	140	139	61	23
	2600	172	11	84	308	144	164	80	30
	3000	171	6	109	350	148	202	93	34
	3500	168	25	144	398	152	246	102	38
	4000	164	35	214	500	156	344	130	48
	5000	156	70	294	617	162	455	161	59
	6000	147	80	404	745	168	567	163	61
	7000	135	110	544		170	730	186	69
	7400		140						
	60000	115			9000				
	7400	60			1600				
	8000				270-227				
			1350	1750		540	2100	350	129

Ultimate 9000 1b

4F 17,500

1	2	3	6	7	8	4	5	9	10
2500		0	35						
7500		112	50						
10000		168	76						
15000		309	124						
17500		378	163						
	200	377	186	165	21	8	8	13	5
	400	376	213	168	45	15	16	29	11
	600	275	226	170	56	8	29	32	12
	800								
	1000	373	255	172	73	16	40	33	12
	1200	372	270	174	96	8	48	48	18
	1400	371	286	175	111	8	56	55	20
	1600	369.5	302	177	125	13	69	56	21
	1800	368	320	179	141	13	82	49	18
	2000	367	337	180	157	8	90	67	25
	2200	365.5	355	182	173	13	103	70	26
	2600	362.5	392	186	206	26	129	77	29
	3000	359	432	190	242	31	160	82	30
	3500	355	481	194	289	35	195	92	34
	4000	351	523	198	325	35	230	95	35
	5000	340	655	206	449	100	330	119	44
	6000	327	800	212	588	120	450	138	51
	7000	310	1095	218	777	160	610	167	62
	8000	262	1500	221	1280	470	1080	200	74
	9000	140	2800	224	2575	1220	2300	275	102

Ultimate 1174016 Sudden broke

1	2	3	4	5F0	6	7	8	9	10
				<u>5</u>					
	200				10	8	2		
	400				15	10	5		
	600				19.5	12	7.5		
	800				27	14	13		
	1000				35	16	19		
	1200				43	18	25		
	1300				59	20	39		

Ultimate 1300 1p

1J0

1	2	3	4	5	6	7	8	9	10
0	0			0	0				
0	200				21	80	13		
0	400				38	10	28		

Ultimate load 40016

1J 2500	19	21	23	25	27	29	37	38
2500								
2500	200							
2500	400							
2500	600							
2500	800							
2500	1000							
2500	1800							
2500	2000							

Ultimate Load 2000 1b

1J 5000	65	85	100	120	146	176	258	368	530	800	1500
2500											
5000	0										
5000	200										
5000	400										
5000	600										
5000	800										
5000	1000										
5000	1400										
5000	1800										
5000	2200										
5000	2600										
5000	8000										

Ultimate load 3200 1b

<u>1 J 7500</u>									
1	2	3	4	5	6	7	8	9	10
2500	0	140			33				
5000	0	227			66				
7500	0	331			115				
7500	200	331			134	117	17-	17	6
7500	400	330	8		154	119	35-	27	10
7500	600	328.5	13	21	180	121	59	38	14
7500	800	326	23	44	213	123	90	46	17
7500	1000	323	28	72	254	125	129	57	21
7500	1400	316	66	138	337	127	210	72	27
7500	1800	306	96	234	453	131	321	87	32
7500	2200	295	96	330	585	135	450	120	44
7500	2600	283.5	116	446	726	139	587	141	52
7500	3000	267.5-280	156	602	911-917	149	768	166	61
7500	3500	262	170	772	1155	53	1002	232	86
7500	4000	232	290	1062	1500	157	1350	288	130
7500	4800	111	1200	2262	4840	162	2778	516	190

Ultimate load 4820 lb

<u>1 J 10000</u>									
1	2	3	4	5	6	7	8	9	10
2500	0	157			27				
5000	0	242			57				
10000	0	406.5			151				
10000	200								
10000	400	406.2	1	1	187	155	32-	31	12
10000	600	405.5	5	6	204	157	47-	41	15
10000	800	404.5-412	8	14	234-238	163	75	61	23
10000	1000	413	8	22	283	165	118	96	35
10000	1400	408	46	68	355	169	186	118	44
10000	1800	401	66	134	143	173	270	136	50
10000	2200	394	66	200	536	171	359	159	59
10000	2600	385	86	286	638	181	447	161	61
10000	3000	376-383.5	80	372	750-754	189	565	193	71
10000	3500	375	80	452	915	193	722	270	100
10000	4000	361	135	587	1081	197	873	306	113
10000	5000	315	450	1037	1575	205	1370	333	124
10000	6000	220	940	1977	2585	211	2316	339	125

Ultimate Load 6400 lb

1	2	3	4	5	6	7	8	9	10
2500	0	26			20				
5000	0	172			40				
7500	0	256			66				
10000	0	331			104				
12500	0	405.5			144				
12500	600	404	9	9	231	150	81	72	26
12500	800	403.5	3	12	242	152	90	78	28
12500	1000	402	13	25	261-267	154	117	82	30
12500	1200	410	8	33	307	158	145	112	41
12500	1400	409	8	41	332	160	168	127	46
12500	1600	406	28	69	370	162	204	135	59
12500	1800	403	28	97	406	164	238	141	52
12500	2000	401	18	115	446	166	276	161	59
12500	2400	394	66	181	530	174	356	175	64
12500	2800	388	56	237	600	178	412	175	64
12500	3000	383-389	48	285	665-669	284	485	200	73
12500	3500	383	55	340	800	288	612	262	96
12500	4000	370	125	465	950	292	760	295	106
12500	5000	545	240	705	1250	200	1060	355	130
12500	6000	313	310	1015	2000	206	1710	695	153
12500	7000	267	450	1465	2260	212	2040	575	210
12500	7400	218	480	1945	2800	213	2600	655	239

Ultimate load 7400 lb

1J 15000

2500	0	73			32				
5000	0	159			64				
10000	0	324			166				
15000	0	477			314				
	200								
15000	400	477			342				
15000	600	476.5	3	3	357	320	37-	34	13
15000	800	476	3	6	380	322	58-	52	19
15000	1000	474	16	22	411	324	87	65	24
15000	1400	469	46	68	477	228	149	81	30
15000	1800	464-466	46	114	550-554	330	214	100	37
15000	2200	462	36	150	620	338	282	132	49
15000	2600	458	36	186	710	342	368	182	67
15000	3000	452	54	240	790	346	454	214	78
15000	3500	443	85	325	910	350	560	235	87
15000	4000	433-443	95	420	1028-1033	359	675	255	94
15000	5000	423	190	610	1290	367	923	313	115
15000	6000	403	190	800	1575	373	1202	402	148
			COMIT	ON NEXT PAGE					

1	2	3	4	5	6	7	8	9	10
15000	7000	371-380	310	1110	1921-1933	383	1550	440	163
15000	8000	347	320	1430	2340	389	1860	430	159
15000	8600				3000				

Ultimate load 8600 lb

1j 17500

2500	0	106				26			
5000	0	183				48			
10000	0	349				140			
15000	0	493				270			
17500	0	558				339			
17500	200								
17500	400	556.5	3	3		371	343	28	25
17500	600	556.5	8	11		387	345	42-	31
17500	800	554	13	24		409	347	62-	28
17500	1000	552.5	13	37		436	349	87	50
1400	1400	548	41	78		502	353	149	71
1800	1800	543.5-550	41	119		565-570	362	208	89
2200	2200	545	46	165		638	366	272	107
2600	2600	538	65	231		718	370	348	117
3000	3000	531	66	297		789	374	415	118
3500	3500	521	95	392		891	378	513	121
4000	4000	510	105	497		998	382	616	119
5000	5000	487	220	717		1235	390	845	128
6000	6000	462	240	957		1490	396	1100	143
7000	7000	430	310	1267		1800	402	1400	133
8000	8000	387	420	1687		2444	408	2040	353

Ultimate load 8600 lb

2j0

0	0	0				0	8	8	
0	200					16	10	14	
0	400					24	12	22	
0	500					34			

Ultimate load 500 lb

<u>2J 2500</u>									
1	2	3	4	5	6	7	8	9	10
2500		82			20	22	7	7	3
2500	200	85			29	24	14	14	5
2500	400	85			38	26			
2500	600	84.5	3	3					
2500	800	83.5	8	11	61	28	33-	22	8
2500	1000	82.5	8	12	76	30	46-	27	10
2500	1200	81.5	8	27	92	32	60	33	12
2500	1400	80.5	8	35	111	34	77	42	15
2500	1600	78.5-86	18	53	136-139	39	100	47	17
2500	1800	84.5	13	66	174	41	133	67	25
2500	2000	81.5	28	94	218	43	175	81	30
2500	2200	76	53	147	278	45	273	126	47
2500	2400	65	108	255	403	47	356	101	27
2500	2460	10	5550	805	1000		950	400	148

Ultimate load 2460 lb

<u>2j 5000</u>									
1	2	3	4	5	6	7	8	9	10
2500		95			20	51	25	17	6
5000		180.5	8	8	49	53	36	20	7
5000	200	178.5	8	16	765	53	53	30	11
5000	400	177.5	13	23	89	57	66	30	11
5000	600	176	13	36	106	59	87	33	12
5000	800	174.5	18	54	123	64	107	35	13
5000	1000	173	18	72	146	66	137	52	19
5000	1200	171-176.5	13	85	168-171	68	168	55	20
5000	1400	175	18	103	203	70	208	72	27
5000	1600	173	33	136	236	72	255	81	30
5000	1800	169.5	38	174	278	80	404	139	41
5000	2000	165.5	91	265	327	88	502	141	52
5000	2400	156-165	96	361	480-484	90	785	166	61
5000	2800	155	258	619	590	92	1048	161	60
5000	3000	129	268	887	875	94	1746	181	67
5000	3200	102	678	1565	1150				
5000	3460	34			1860				

Ultimate Load 3460 lb

1	2	3	4	5	6	7	8	9	10
2500	0	100			20				
5000	0	191			50				
7500	0	274			78				
7500	200	273.5-283.5	3	3	90-94	84	10	7	3
7500	400	288.5	8	11	113	86	17	6	6
7500	600	281.5	8	19	131	88	43	24	9
7500	800	280	13	32	151	90	61	29	11
7500	1000	278	18	50	174	92	82	32	12
7500	1200	276	18	68	203	94	109	41	15
7500	1400	274	18	86	232	96	136	50	18
7500	1600	271	28	114	266	98	168	54	80
7500	1800								
7500	2000	265	56	170	346	102	245	75	28
7500	2200	265			404	104	300		
7500	2400	262	28	198	451	106	355	157	58
7500	2600								
7500	2800	251	106	304	578	140	468	164	61
7500	3000	245	56	360	652	112-119	540	180	67
7500	3500	225	234.5-190	550	890-	893	780	230	85
7500	4000	211	230	780	1210	123	1087	307	112
					2800				

Ultimate load 4640 lb

2J 10000

2500	0	100			20				
5000	0	187			40				
7500	0	271			70				
10000	0	349			108				
10000	200	349			120				
10000	400	348.5	3	3	130	110	10	10	4
10000	600	7.5	48	11	145	112	18	15	6
10000	800	6.5	8	19	165	114	31	20	7
10000	1000	356-351	8	27	186-190	116	59	40	15
10000	1200	350	8	35	222	122	78	51	19
10000	1400	349	8	43	249	124	98	63	23
10000	1600	347.5	13	66	270	126	118	75	28
10000	1800	345	23	89	305	128	142	76	28
10000	2000	342.5	23	112	340	130	175	86	32
10000	2400	336.5	56	168	415	132	218	106	39
10000	2600	333	33	201	453	136	279	111	41
10000	2800	329	38	239	500	138	315	114	42
Con't. on next page.						140	360	121	44

<u>2J 1000</u>										
1	2	3	4	5	6	7	8	9	10	
2500	0	123			28					
5000	0	230			48					
12500	0	455			134					
15000	0	531			192					
15000	200	531		3	206	174	12	12	4	
15000	400	530.5	3	11	223	196	27	24	9	
15000	600	529.5	8	24	244	198	46	33	12	
15000	800	528	13	37	261	200	61	37	14	
15000	1000	526.5	13	50	281	202	79	42	15	
15000	1200	325	13	68	304	204	100	50	18	
15000	1400	223	18	86	330	206	124	56	21	
15000	1600	221	18	109	357	208	149	63	23	
15000	1800	218.5	23	127	387	210	177	68	25	
15000	2000	216.5	18	145	418	212	197	70	26	
15000	2200	214.5-223.5	18	158	440-455	220	235	90	33	
15000	2400	222	13	171	508	222	286	128	37	
15000	2600	220.5	13	189	537	224	313	142	47	
15000	2800	218.5	18	212	570	226	344	155	53	
15000	3000	516	23	287	603	228	375	163	57	
15000	3500	508	75	362	704	232	462	175	60	
15000	4000	500	75	552	807	236	571	209	65	
15000	5000	480	190	822	1055	244	811	259	77	
15000	6000	462	270	1132	1350	250	1100	278	95	
15000	7000	420	310	1802	1736	256	1480	348	103	
15000	8000	352	670		2400	262	2150	348	128	
15000	8200				2800				128	

Ultimate load 8300 lb

I	Z	J	4	3	6	I	8	9	10
2500	0	87			18				
5000	0	174			39				
7500	0	259			68				
10000	0	341			107				
12500	0	401			150				
15000	0	476			206				
17500	0	545			268				
17500	200	545			273				
	400	545	3	3	281				
	600	544.5	3	6	292				
17500	800	543	13	19	303				
17500	1000	541.5	13	32	336				
17500	1200	540	13	45	360				
17500	1400	537.5	23	68	390				
17500	1600	536	13	81	414				
17500	1800	533.5	23	104	445				
17500	2000	531.5	23	127	472				
17500	2200	5285	23	150	507				
17500	2400	526.5	23	173	535				
17500	2600	523.5	23	196	569				
17500	2800	520	31	227	605				
17500	3000	516	35	262	645				
17500	3500	508-517	75	337	740				
17500	4000	508	85	422	835				
17500	5000	487	200	622	1043				
17500	6000	464	220	842	1279				
17500	7000	442	210	1052	1542				
17500	8000	397	440	1492	1960				
17500	9000	315	780	2272	2800				

Ultimate load 9200 lb

3J 2500

2500	0	10							
2500	200	18							
2500	400	26							
2500	600	36							
2500	800	48							
2500	1000	61							
2500	1200	80							
2500	1400	102							
2500	1600	124							
2500	1800	153							
2500	2000	200							
2500	2200	362							

Ultimate Load 2200 lb

1	2	3	4	5	6	7	8	9	10
0	200				15	8	7		
0	400				26	10	16		
0	600				38	12	26		
0	620				52	14	38		
0									

Ultimate load 620 lb

3J 5000

2500	0	107			20				
5000	0	203.5			45				
5000	200	202.5	13	13	60	47	13	0	
5000	400	201.5	13	26	73	47	24	0	
5000	600	200-211	13	39	88-90	53	37	0	
5000	800	210	8	47	111	55	56	9	3
5000	1000	209	8	55	130	57	73	18	7
5000	1200	207.5	13	68	152	69	93	25	9
5000	1400	206	13	81	177	61	116	25	9
5000	1600	203.5	23	104	210	63	147	43	16
5000	1800	200	38	137	252	65	187	50	19
5000	2000	184	150	287	500	67	433	246	97

Oil got in specimen
Ultimate load 2000 lb

3J 10000

2500	0	60			20				
5000	0	160			50				
7500	0	233			78				
10000		312.5			119				
10000	200								
10000	400	312	3	3	142	123	9	6	2
10000	600	311	8	11	159	123	26	15	6
10000	800	310	8	19	178	127	51	32	12
10000	1000	308	18	37	208	129	89	52	19
10000	1200	306-310	18	55	227-230	135	95	40	15
10000	1400	309	8	43	271	137	134	91	33
10000	1600	307	18	61	300	139	160	99	37
10000	1800	304.5	23	84	338	141	197	113	42

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10000	2600	287	43	101	313	143	230	123	45
10000	3000	283-288	44	151	511	149	262	111	41
10000	3500	280	36	187	611-631	170	461	174	64
10000	4000	272	35	262	771	174	597	325	120
10000	5000	230-235	75	337	928	178	760	423	154
10000	6000	267	410	747	1440-1450	195	1250	503	176
					1230	201	1030		

Ultimate load 6000 lb

3J 12500

2500	0	244		24		97	23
5000	0	330		36		99	33
7500	0	412		65		101	63
10000	0	487		88		103	88
12500	0	550		95		105	110
12500	200	564		120		107	136
12500	400	564		132		109	168
12500	600	564		164		111	190
12500	800	564		191		113	230
12500	1000	564		215		115	279
12500	1200	564		243		117	311
12500	1400	563.8		277		119	380
12500	1600	563		301			
12500	1800	562		343			
12500	2000	561		394			
12500	2200	559.5		428			
12500	2400	557.5		499			

Oil got in Ultimate load 2420 lb

3J 15000

2500	0	52		20		211	8	5
5000	0	147		43		213	15-	9
7500	0	216		68		215	28-	19
10000	0	293		104		217	47	25
12500	0	365		150		224	73	38
15000	0	434		209		226	110	62
15000	200	434.5	3	219				
15000	400	434	3	228				
15000	600	433.5	3	243				
15000	800	432	13	264				
15000	1000	430.5-437	13	292-297				
15000	1200	435.5	13	336				

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1	2	3	4	5	6	7	8	9	10
15000	1400	434	13	61	365	228	137	66	24
	1600	431.5	23	84	398	231	167	83	31
15000	1800	429	23	107	428	233	195	88	33
	2200	423.5	61	168	505	238	268	100	37
	2600	417	61	229	582	241	341	112	41
	3000	411-416	56	235	665-668	245	423	188	69
15000	3500	409	65	300	782	249	533	233	86
	4000	401	75	375	900	253	647	272	100
	5000	379	210	585	1100	261	3840	255	94
	6000	353-370	250	835	1460-465	267	1200	365	135
15000	7000	340	290	1125	1890	273	1620	495	183
15000	8000	300	390	1515	2470	274	2200	685	253

Ultimate load 8450 lb

3J 17500

2500	0				40				
5000	0				73				
7500	0	93			108				
1000	0	162			159				
12500	0	232			210				
15000	0	309			270				
17500	0	360							
17500	200	359			290	274	16	0	1
	400	358			304	276	28-	4	6
17500	600	357	8	16	325	278	47-	15	
	800	356	38	24	350	280	70	30	
17500	1000	355	8	40	380	282	98	50	
	1200	354-360	8	48	430	284	146	75	
	1400	358.5	13	61	459	286	173	99	
	1600	357	13	74	486	288	198	106	
	1800	355	18	92	522	290	222	112	
17500	2000	353	18	110	554	292	262	134	
17500	2200	351	18	128	627	296	331	147	
	2600	345	56	184	703	300	400	172	
	3000	340	44	228	804	304	500	187	
	3500	331	85	313	910	308	600	202	
17500	4000	322	85	398	1145	316	829	341	
	5000	312	90	488	1390	322	978	378	
	6000	299	120	600	1680	328	1352	492	
17500	7000	272	260	860	2250	330	1920	720	
	8600	237	340	1200	2600	333	2266	696	
17500	9200	200	370	1570					
			Ultimate load 9200 lb						

1	2	3	4	5	6	7	8	9	10
0	200				16	8	8		
0	400				27	10	17		
0	430				38	12	26		

Ultimate load 430 lb

<u>4J 5000</u>									
2500	0	110			20				
5000	0	199			50				
	200	198.5	3	3	65	52	13	10	4
5000	400	197.5	8	11	76	54	22	11	4
	600	196.5	13	24	90	56	34	10	4
5000	800	195	8	32	107	58	49	17	6
5000	1000	172.5-204.5	21	53	126	60	66	13	5
5000	1200	202	23	76	160	62	98	22	8
5000	1400	200	18	94	192	64	128	34	13
5000	1600	198	18	112	223	66	157	45	17
5000	1800	194	38	150	270	68	202	52	19
	2200	184	98	248	387	70	317	69	26
5000	2600	171	128	376	581	74	506	130	49
5000	3200	72	986	1362	1600	78	1522	160	60

Ultimate load 3300 lb

<u>4J 15000</u>									
2500	0	62			20				
5000	0	148			40				
7500	0								
10000	0	303			111				
12500	0	382			168				
15000	0	452			224				
15000	200	452			244	226	18		
	400	451	8		256	228	28	20	7
15000	600	450	8		270	230	40	24	9
15000	800	448.5	13	8	291	232	59	30	11
	1000	447	13	42	317	234	73	31	11
	1200	446-450	8	50	345-351	252	99	49	17
15000	1400	448.5	13	63	389	254	135	72	27
	1600	447	13	76	417	256	161	85	31
15000	1800	445	18	94	450	258	192	98	36
	2200	439	54	149	550	262	288	139	51

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4J 12000 CONL'D.

1	2	3	4	5	6	7	8	9	10
15000	2600	334.41	46	195	594-600	272	328	133	49
15000	3000	437.5	31	226	695	276	420	194	72
15000	3500	430.5	70	296	790	280	510	214	79
15000	4000	421.1	85	381	900	284	616	235	87
15000	5000	400-405	205	586	1141-1148	277	652	206	98
15000	6000	380	180	766	1440	305	1145	379	140
15000	7000	353	326	1020	1820	311	1509	489	180
15000	8000	396-414			2500	317	2200		

Ultimate 8400 lb

1H0

0	200	10	8	2
0	400	15	10	5
0	600	19	12	7
0	800	27	14	13

Ultimate load 800 lb

1H 2500

2500	0	127	8	8	26	28	8	0	0
2500	200	126	8	16	36	30	17	1	0
2500	400	125	8	24	47	32	28	4	1
2500	600	124	8	32	60	34	40	8	2
2500	800	123	8	45	74	37	53	8	2
2500	1000	121.5-126	13	61	89-90	39	90	27	13
2500	1400	124	16	102	129	41	137	35	13
2500	1800	119.5	41	163	178	43	212	49	18
2500	2200	113	61	289	255	45	353	64	23
2500	2600	100	126	616	398	46	154	38	24
2500	2800	67	327		700				

Ultimate load 2820 lb

IH 10000

1	2	3	4	5	6	7	8	9	10
2500	0	81			2.0				
5000	0	167			35				
7500	0	258			57				
10000	0	343.5			92				
10000	200	343	3	3	105	94	11	8	3
10000	400	342.5	3	6	112	96	16	10	4
10000	600	341.5	8	14	122	98	24	10	4
10000	800	340	13	37	139	100	39	2	1
10000	1000	338.5-341.5	13	50	153-155	104	51	1	0
10000	1400	340	11	61	195	108	87	26	9
10000	1800	337.5	21	82	235	112	123	41	15
10000	2200	334.5	26	108	280	116	164	56	20
10000	2600	331	31	139	326	120	206	67	24
10000	3000	327-331	36	175	376-378	126	252	77	28
10000	3500	327.5	30	205	444	130	314	109	39
10000	4000	322	50	255	511	134	377	122	44
10000	5000	308-312	130	385	668-671	145	526	141	51
10000	6000	896	150	535	887	161	726	191	70
10000	6500	375	205	740	1100	169	940	200	73

Ultimate load 6600 lb

IH 7500

1	2	3	4	5	6	7	8	9	10
2500	0	92			13				
7500	0	251			47				
7500	200	250	8	8	62	49	13	5	2
7500	400	249	8	16	75	51	24	8	3
7500	600	248	8	24	89	53	36	12	4
7500	800	247	8	32	100	55	45	13	5
7500	1000	246-250	8	40	122-124	59	65	25	9
7500	1400	248	16	56	165	61	104	48	17
7500	1800	245	26	82	215	63	-152	60	22
7500	2200	240	46	128	265	67	198	70	26
7500	2600	235	46	174	326	71	255	81	30
7500	3000	230-238.5	46	220	380-392	76	316	96	35
7500	3500	334	46	260	481	80	401	141	51
7500	4000	325	85	345	580	84	496	151	55
7500	5000	299	250	590	887	92	795	205	75

Ultimate Load 5280 lb

1H 15000									
1	2	3	4	5	6	7	8	9	10
2500	0	187			18				
5000	0	269			41				
10000	0	436			107				
12500	0	512			141				
15000	0	572			175				
	200	571.5	3	3	181	177	4	1	0
15000	400	570.5	8	11	190	179	11	0	
15000	600	569.5	8	19	200	181	19	0	
15000	1000	566.7	23	42	223	183	43	1	0
15000	1400	563.7	26	68	262	185	77	9	3
	1800	560.3	30	98	297	187	110	12	4
15000	2200	556.3	36	134	337	191	136	2	1
	2600	552	39	173	381	195	186	13	5
15000	3000	546	36	209	414	199	215	16	6
	3500	547	4	213	470	203	267	54	20
15000	4000	544	30	243	527	207	320	75	28
	5000	535.5	85	328	643	215	428	100	37
15000	6000	526.5	80	408	779	221	558	150	56
	7000	513.7	128	536	962	226	736	200	74
15000	7800				1265	230	1035		

Ultimate load 7970 lb									
1H 12500									
2500	0	167	20						
10000	0	416	83						
12500	0	495	113						
12500	200	494.5	130	115	15	12	4		
	400	494	143	117	26	20	7		
12500	600	493.5	157	119	38	29	10		
	1000	492.5-494.5	189-192	124	66	51	18		
12500	1400	493	224	128	96	70	25		
	1800	490	257	136	121	79	29		
12500	2200	488	298	142	156	88	33		
	2600	484.5	335	148	187	88	33		
12500	3000	481.5	376	152	214	89	33		
	3500	478.5-473	432-435	160	275	105	38		
12500	4000	460.5	488	164	324	84	31		
	5000	472	602	172	430	155	36		
12500	6000	461.5	730	178	552	142	52		
	7000	447	895	194	701	166	61		
2500	7800	417	1200	200	1000	176	64		

1	2	3	4	5	6	7	8	9	10
	8000	431	110	502	991	245	746	244	
	7000	410	200	702	1216	251	965	263	89
	9400	393	166	863	1416	257	1256	388	96
									142

Ultimate load 9600 lb

2H0

0	0
0	200
0	400
0	520

0
17
25
35

8
10
12

9
15
23

Ultimate load 520 lb

2H 2500

2500

200	172
400	171.5-181
600	180.5
800	179.5
1000	179
1200	178.5
1400	178
1600	177
1740	175

20
37.5-39
45
54
64
75
87
105
125
170

24	15	12	4
26	19	13	5
28	26	12	4
30	34	17	6
32	43	23	8
34	53	30	11
36	69	38	14
38	87	40	15
40	130		

Ultimate load 1740 lb

2H 5000

2500	10	160
5000	0	243
5000	200	242.5-257
	400	256.5
5000	600	255.5
5000	800	254.
	1000	253
5000	1200	251.5
	1400	250
5000	1600	248

22
43
53-55
68
81
95
112
132
153
176

47	8	5	2
47	19	13	5
51	30	16	5
53	42	5	2
55	57	12	4
57	75	17	6
59	94	23	8
61	115	26	9

144

1	2	3	4	5	6	7	8	9	10
5000	1600	248	18	89	176	61	115	26	10
5000	1800	246	18	107	204	63	141	34	9
5000	2000	242.5-253	33	140	249-251	68	183	43	12
	2200	248	48	188	330	70	260	72	15
	2800	208	394	582	800	76	725	143	26
5000	3000	196	118	700	920	78	847	187	52
	3500	124	715	1415	1800	80	1720	305	66
	3600	25			2800	2700			112

Ultimate 3800 lb

2H 75

2500	0	8		3	20	80	8	5	2
5000	0	75			43	82	16	5	2
7500	0	160.5			78	84	31	12	4
7500	200	160	3	11	88	86	47	20	7
7500	400	159	8	19	98	88	65	30	11
7500	600	158	8	27	115	94	85	37	13
7500	800	157	8	35	133	96	114	53	19
	1000	150	8	48	153	98	136	62	23
7500	1200	154.5-163	13	61	175-179	100	161	69	25
7500	1400	161.5	13	74	210	102	189	79	28
7500	1600	160	13	92	234	104	218	80	29
	1800	158	18	110	261	106	250	84	30
7500	2000	156	18	138	291	108	279	85	31
7500	2200	153	28	166	322	112	344	96	35
7500	2400	151	28	194	356	120	442	89	33
7500	2600	148	28	248	387	124	516	103	37
7500	3000	142.5	54	353	456	132	1200	187	50
	3500	1355-1435	65	413	550-562	135	1965		
	4000	137	60	1063	640				
7500	5000	70	650		1330				
7500	5040	000			2000				

Ultimate load 5040 lb

2H 10

2500	0				20				
5000	0				39				
7500	0				60				
10000	0	351.5			86				
10000	200	351	3	3	98-100	90	10	7	3

1	2	3	4	5	6	7	8	9	10
10000	400	550.5-358	3	6	113	92	21	15	5
10000	600	358.5	3	9	124	94	30	21	7
10000	800	359	3	12	136	96	40	28	10
10000	1000	358	8	20	152	98	54	34	12
10000	1200	357.5	3	23	165	100	65	42	15
	1400	356.5	8	31	182	102	80	49	18
	1600	355.5	8	39	200	104	96	57	21
10000	1800	354	13	52	218	106	112	60	22
10000	2000	352.5-362.5	13	65	239-242	108	134	69	25
10000	2200	361	13	78	268	110	158	80	29
	2400	359.5	13	91	290	112	178	87	32
10000	2600	358.5	8	98	312	114	198	99	36
	2800	357	13	112	336	116	220	108	38
10000	3000	355.5	13	125	359	118	241	116	42
	3500	349	55	180	435	122	313	133	48
10000	3800	335	137	317	700	127	575	258	54

Oil gets in Ultimate load 3800 lb

2H 12500

2500	0	82			20				6
5000	0	170			36				11
7500	0	251			61				14
10000	0	327			93				16
12600	0	402			136				19
12600	400	401-410.5	116	6	162-167	145	22	16	20
	600	411	3	9	185	147	38	29	23
	800	410.5	3	12	200	149	51	39	29
12600	1000	409.5	8	20	215	151	64	44	31
	1200	408.5	8	28	234	153	81	53	33
12600	1400	407	13	41	252	155	97	56	36
	1600	405.5	13	53	272	157	115	62	40
	2000	402.5	26	79	317	159	158	62	62
	2200	401	13	92	330	161	169	79	20
	2400	399.5	13	105	352	163	189	77	23
	2600	398	13	118	374	165	209	84	28
	3000	393.5	41	159	425	167	258	91	31
	3500	388	50	209	490	171	319	92	33
12600	4000	383-387	45	214	555-558	175	383	110	36
	5000	380	65	279	690	183	507	170	40
12600	6000	364	155	434	870	101	680	228	62
	7000	329-333	340	774	1257-1267	199	1070	246	83
	7200	300	328	1102	1700	200	1500	296	91
		Ultimate load 7200 lb						400	108
									146

1	2	3	4	5	6	7	8	9	10
2500	0	45			20				
5000	0	130			40				
7500	0	212			60				
10000	0								
12500	0	363			121				
15000	0	440			159				
	400	339	3	3	172	163	9	6	2
	600	338.5	3	6	180	165	15	9	3
	800	338	3	9	192	167	25	16	5
	1000	337	8	17	209	169	40	23	8
	1200	336	8	25	221	171	50	25	9
	1400	335	8	33	237	173	64	31	11
	1600	333.5	13	46	254	175	79	33	13
	2200	329	39	85	310	179	131	46	17
	2600	326.5	21	106	345	183	162	56	21
	3000	323	31	137	387	187	200	63	23
	3600	317	49	186	452	191	261	75	27
	4000	314	26	212	403	195	298	86	31
	5000	305-311	85	297	615-625	213	412	115	42
	6000	302	85	382	750	219	531	149	5
	8000	287	140	522	1120	225	895	373	13
	8600	231	540	1062	1500	227	1275	213	79

ULTIMATE LOAD 8600 lb

2H-17500

10000	0	65			107				
12500	0	145			140				
15000	0	222			170				
17500	0	268			201				
	200	268	0	0					
	400	268.5	3	3	220	205	15	12	4
	600	268	3	6	228	207	21	15	5
	800	267.5	3	9	240	209	31	22	8
	1000	267	3	12	252	211	41	29	11
	1200	266	8	20	268	213	55	35	13
	1400	265	8	28	282	215	67	39	14
	1600	264	8	36	298	217	81	45	16
	1800	263	8	44	314	219	95	51	18
	2200	260	26	70	350	225	125	55	20
	2600	257.5	21	91	384	228	156	65	23
	3000	255	21	112	418	233	175	63	23
	3500	258	25	137	475	237	238	101	37

1	2	3	4	5	6	7	8	9	10
	4000	256	25	162	520	241	279	117	43
	5000	249	60	222	610	249	361	139	51
	6000	240	80	302	720	255	465	163	59
	7000	233.5	55	357	800	261	539	182	67
	8000	230	25	382	955	267	638	306	112
	9000	213	160	542	1130	270	960	418	153
	9650	180	325	867	1500	273	1225	458	1660

ULTIMATE LOAD 9650 lb

3H 5000

2500	0	200			20				
5000	0	288			44				
5000	200	288	0		52	46	6	6	2
	400	287.5	3	3	59	48	11	8	3
5000	600	286.5	8	11	69	50	19	8	3
5000	800	286	3	14	80	52	28	14	5
5000	1000	285	8	22	91	54	37	15	6
	1200	284	8	30	107	56	51	21	8
5000	1400	283	8	38	122	58	64	26	9
	1600	282	8	46	130	60	79	33	12
5000	1800	280-288	16	62	159-161	65	96	34	12
	2200	285	26	88	209	69	140	52	19
5000	2600	282	26	114	261	73	188	74	27
	3000	276	56	170	328	77	251	81	27
	3500	265	105	275	450	81	369	94	34
5000	3900	250	145	420	608	85	515	105	38

ULTIMATE LOAD 3900 LB

3H 7500

2500	0	4			20				
5000	0	98			43				
7500	0	186			74				
	200	185	8	8	87	76	11	3	1
7500	400	184	8	16	98	78	20	4	1
	600	182.5	13	23	114	80	34	11	4
	800	181	13	36	130	82	48	12	4
7500	1000	179	8	44	151	84	67	23	8
7500	1200	176.5	23	67	174	86	88	21	8
	1400	174.5	18	85	198	88	110	25	9
	1600	173-183.5	13	98	222-225	93	133	35	13

3H 7500 Cont'd.

1	2	3	4	5	6	7	8	9	10
7500	1800	182	13	111	262	75	167	56	20
	2200	177	46	157	320	77	223	66	24
7500	2600	172	46	203	384	101	280	77	28
	3000	166	56	259	455	105	350	91	33
7500	3500	155	105	364	557	109	448	84	31
	4000	145	95	459	685	114	571	112	41
	5000	59	850	1309	1580	117	1473	164	60

ULTIMATE LOAD 5200 lb

3H 10000

2500	0	98			20				1
5000	0	183			42				2
7500	0	260			64				4
10000	0	346			100				8
	200	345.5	3	3	107	102	5	2	12
10000	400	345	3	6	114	104	10	4	15
	600	344.5	3	9	127	106	21	12	16
10000	800	344	3	12	142	108	34	22	19
	1000	343	3	15	159	110	49	34	28
	1200	341.5	8	23	177	112	65	42	39
	1400	340	13	36	195	114	81	45	42
	1600	398.5	13	49	219-222	119	103	54	45
	1800	343.5-344.5	8	57	251	121	130	77	49
10000	2200					125			59
	2600	338	47	104	340	129	211	107	73
	3000	334	36	140	389	134	255	115	
10000	3500	328	55	195	457	138	319	124	
	4000	321	65	260	537	142	395	135	
	5000	306	140	400	711	150	561	161	
10000	5980	276-290	290	690	1042-1047	158	890	200	

Ultimate Load 5980

1	2	3	4	5	6	7	8	9	10
2500	0	175.5	3	20	22				
2500	200	175	3	27	24				
2500	400	174.5	6	35	26				
2500	600	174	9	44	28				
2500	800	173.5	12	54	30				
2500	1000	172	15	65	32				
2500	1200	171	28	30	34				
2500	1400	170	36	95	36				
2500	1600	179-180	44	114-116	38				
2500	1800	178	52	146	42				
2500	2200	172	108	225	46				
2500	2400	159	234	359	48				
2500	2000	120	624	750					

Ultimate Load 2400 lb

3H 12500

2500	0	80	-	20	123				
5000	0	173	-	40	125				
7500	0	258	-	60	127				
10000	0	327	3	84	129				
12500	0	407	6	121	131				
200	200	407	14	127	133				
400	400	407	22	135	135				
600	600	406.5	30	145	137				
800	800	406	38	158	132				
1000	1000	405	51	171	147				
1200	1200	404	75	186	148				
1400	1400	403	91	201	153				
1600	1600	402	107	219	157				
1800	1800	400.5	187	238	161				
2400	2400	397.5-404	24	275-279	169				
2600	2600	402	16	322	175				
3000	3000	400	16	362	181				
3500	3500	396	35	416	184				
4000	4000	391	45	472					
5000	5000	380-388	100	610-613					
6000	6000	379	80	762					
7000	7000	361	160	960					
7580	7580	314	480	1440					

Ultimate Load 7580 lb.

4H 10000

1	2	3	4	5	6	7	8	9	10
2500	0	166	0		20				
5000	0	249			39				
7500	0								
10000	0	413.5	0	0		115	6	3	1
10000	200	413.5	3	3	123	117	10	4	2
10000	400	413	3	6	129	119	17	8	3
10000	600	412.5	3	9	138	121	17		
10000	800	412	3	12	152	123	29	17	
10000	1000	411.5	3	20	170	125	45	17	
10000	1200	410.5	8	23	185	127	55	25	6
10000	1400	410	3	31	204	129	75	35	9
10000	1600	409	8	49	221	131	90	44	15
10000	1800	407	18	62	242	133	109	41	16
10000	2000	405.5	13	80	263-267	135	132	47	16
10000	2200	403.5-408	18	96	322	141	181	52	17
10000	2600	406	16	145	433	154	279	85	19
10000	3500	400	49	200	502-506	162	344	134	31
10000	4000	394-398	55	280	675	168	507	144	49
10000	5000	389	80	470	880	174	706	227	53
10000	6000	369	190	1100	1530	177	1370	236	83
10000	6300	305	630					270	85
									99

Ultimate load 6300 lb

5H0

0									
	200				15	8	7		
	400				27	10	17		

Ultimate load 430 lb

1C0

1	2	3	4	5	6	7	8	9	10
0	200				19	8	11		
0	400				27	10	17		
0	600				37	12	25		
0	800				46	14	32		

Ultimate load 800 lb

Broke into three pieces Angles 28° and 16°

1C 2500

2500	0	292			15	17	15		
2500	200	293.5			32	19	26		
	400	292.5			45	21	37		
2500	600	291.5			58	23	45		
2500	800	290.5			68	25	54		
	1000	289			79	27	65		
2500	1200	287.5			92	29	75		
	1400	286			104	31	90		
	1450	284.5			121				

Ultimate load 1450 lb

Broke into two pieces some longitudinal splitting. Angle 31°

1C 5000

1	2	3	4	5	6	7	8	9	10
2500	0	130			14				
5000	0	228			20				
5000	200	227			28	22	6		
5000	400	226			43	24	19		
	600	225			53	26	27		
5000	800	224			64	28	36		
	1000	223			74	30	44		
	1200	221.5			85	32	53		
	1400	220.5			96	34	62		
5000	1600	219			112	36	76		

Ultimate load 1760 lb

Mostly longitudinal splitting Angle 22°

1C 7500

2500	0	339			5				
5000	0	416			10				
7500	0	519.5			17				
7500	200	518.5			35	21	15		
7500	400	517.5			44	23	21		
	600	516.5			54	25	29		
7500	800	515.5			64	27	38		
	1000	514.5			75	29	46		
	1200	513			85	31	54		
7500	1400	511.5			98	33	65		
7500	1600	510			113	35	78		

Ultimate load 1600 lb

Piece Damaged

<u>IC 10000</u>									
1	2	3	4	5	6	7	8	9	10
0									
2500	0	140			14				
5000	0	276			21				
7500	0	362			25				
10000	0	445			34				
10000		534							
10000	200	533.5	3	3		36			
10000	400	532.5	8	11	53	38	15	4	1
10000	600	531.5	8	19	62	40	22	3	1
	800	530.5	8	27	76	42	34	7	3
	1000	529.5	8	35	86	44	42	7	3
10000	1200	528.5	8	43	98	46	52	9	3
	1400	527.5	8	51	113	48	65	14	5
10000	1500	526	13	64	130	50	80	16	6

Ultimate load 1520 lb

Piece damaged

<u>IC 12500</u>									
1	2	3	4	5	6	7	8	9	10
2500	0	217			11				
5000	0	300			19				
7500	0	390			24				
10000	0	471			32				
12500	0	553.5			41				
	200	553			52	43	9	9	3
	400	552.5	8	8	64	45	18	10	4
	600	551.5	8	16	75	47	28	12	4
	800	551	3	19	85	49	36	17	6
12500	1000	550	8	27	95	51	44	17	6
	1200	449	8	35	105	53	52	17	6
	1220	448	8	43	116	55	61	25	9
	1100	447	8		128	57	71	26	10

Ultimate load 1220 lb

Broke into two pieces Angles 31° and 29°

IC 15000

1	2	3	4	5	6	7	8	9	10
2500	0	167			13				
5000	0	260			18				
7500	0	341			22				
10000	0	428			29				
12500	0	494			34				
15000	0	563			44				
15000	200	563			53	46	7-		
15000	400	562.5	3	3	64	48	16	13	5
15000	600	561.5	8	11	75	50	25	14	5
15000	800	560.5-562.5	8	19	85	52	33-	14	5
15000	1000	563.5	8	27	98	54	44	17	6
15000	1200	563	3	30	107	56	51	21	8
15000	1300	562.5	3	33	117	58	59	26	10
	1350	561.5	8	41	130	60	70	29	11

Ultimate load 1350 lb

IC 17500

1	2	3	4	5	6	7	8	9	10
2500	0	108			13				
5000	0	209			20				
7500	0	286			23				
10000	0	370			30				
12500	0	450			38				
15000	0	507			47				
17500	0	575			56				
	200	574	8	8	70	58	12-	4	1
17500	400	573.5	3	11	80	60	20	9	3
	600	573	3	14	89	62	27	13	5
	800	572	8	22	97	64	33-	11	4
	1000	571	8	30	107	66	41	11	4
	1200	570	8	38	117	68	49	11	4
	1300	569	8	46	128	70	58	12	4

Ultimate load 1300 lb

Broke into two pieces, angle 28° 30 m
Longitudinal splitting.

1	2	3	4	5	<u>2C0</u>	6	7	8	9	10
0	200					16	8	8		
0	400					21	10	11		
0	600					25	12	13		
0	800					29	14	15		
0	1000					34.5	16	18.5		
0	1240					43	18	25		
						<u>Ultimate load 1240 lb.</u>				

Broke into two pieces. Angle 26°

2500					<u>2C2500</u>					
	0					17	19	8		
	200	27				27	21	15		
	400	36				36	23	21		
	600	44				44	25	26		
	800	51				51	27	31		
	1000	58				58				

Ultimate load 1020 lb.

Piece damaged

	<u>2C5000</u>									
1	2	3	4	5	6	7	8	9	10	
2500		44			20					
5000		129			39					
	200				49	41	8			
	400				60	43	17			
	600	126			70	45	25			
	800	125			80	47	33			
	1000	123.5			93	49	44			
	1340	121			108	50	58			

Ultimate load 1340 lb.

Broke into two pieces; some longitudinal splitting. Angle 30°

	<u>2C7500</u>									
2500	0	138			16					
5000	0	229			25					
7500	0	319			33					
	200				43	35	8	8	3	
	400	318.8			51	37	13	13	5	
	600	318	6	6	61	39	21	15	5	
	800	317	8	14	69	41	28	14	5	
	1000	316	8	22	78	43	35	13	5	
	1200	315.5	3	25	88	45	43	18	7	
	1400	315.0	3	28	100	47	53	25	9	
	1600				116	49	67			

Ultimate load 1680 lb.

Broke into two pieces. Angle 34°

2C10, 000

1	2	3	4	5	6	7	8	9	10
2500	0	175			8				
5000	0	271			13				
7500	0	355			16				
10000	0	437			24				
	200	436.5	3	3	34	26	8	6	2
	400	435.5	8	11	45	28	17	6	2
	600	435	3	14	55	30	25	11	4
	800	434	8	22	66	32	34	12	4
	1000	433	8	30	77	34	43	13	5
	1050	432	8	38	90	36	54	16	6

Ultimate load 1050 lb.

Broke into two pieces. Angle 22°. Some longitudinal splitting.

2C12, 500

1	2	3	4	5	6	7	8	9	10
2500	0	185			15				
5000	0	283			22				
7500	0	359			29				
10000	0	445			35				
12500	0	520			43				
	200	521	8	8	59	45	14	6	2
	400	522	8	16	70	47	23	7	3
	600	521.5	3	19	80	49	31	12	4
	800	521	3	22	91	51	40	18	7
	1000	519.5	3	25	104	53	51	26	10
	1200	518	13	38	122	55	67	29	11
	1400	515	28	66	150	57	93	27	10

Ultimate load 1400 lb.

Broke into two pieces. Angle 31° and 30°

2C17, 500

1	2	3	4	5	6	7	8	9	10
2500	0	88			20				
5000	0	177			25				
7500	0	262			32				
10000	0	341			41				
12500	0								
15000	0	482			60				
17500	0	548.5			71				
	200	548	3	3	81	73	8	5	2
	400	547.5	3	6	90	75	15	9	3
	600	546.5	8	14	99	77	22	8	3
	800	555.5	8	22	108	79	29	7	3
	1000	555	3	25	118	81	37	12	4
	1200	554	8	33	127	83	44	11	4
	1400	553	8	41	137	85	52	11	4
	1600	551.5	13	54	150	87	63	9	3
	1800				166	89	77		

Ultimate load 1800 lb.

Piece damaged.

3C0

0	200				14	8	6		
	400				20	10	10		
	600				25	12	13		
	860				30	14	16		

Ultimate load 860 lb.

Broke into two pieces. Some longitudinal splitting 32°.

<u>3C2500</u>										
1	2	3	4	5	6	7	8	9	10	
2500	0	117	8	8	16	18	12	4	1	
	200	116	8	16	30	20	18	2	1	
	400	115	3	19	44	22	22	3	1	
	600	114.6	3	22	49	24	25	3	1	
	800	114	0	22	55	26	29	7	3	
	1000	114	8	30	64	28	36	6	2	
	1200	113	8	38	75	30	46	8	3	
	1400	112	16	56	100	32	68	12	4	
	1200	110								

Ultimate load 1410 lb.

Broke into three pieces, some longitudinal splitting. Angles 32° and 21°.

<u>3C5000</u>										
2500	0	109	0	0	11	24	8	8	3	
5000	0	203	2	2	22	26	17	17	6	
	200	203	0	0	32	28	23	21	8	
	400	202.8	8	10	43	30	28	18	7	
	600	202.5	8	18	51	32	35	17	6	
	800	202.5	8	26	58	34	42	16	6	
	1000	201.5	8		67					
	1200	200.5	8		76					

Ultimate load 1200 lb.

Piece damaged

<u>3C 7500</u>									
1	2	3	4	5	6	7	8	9	10
2500	0	285			16				
5000	0	375			28				
7500	0	468			38				
	200	467			45	40	5		
	400				57	42	15		
	600				67	44	23		
	800				76	46	30		
	1000				85	48	37		
	1200				95	50	45		
	1400				106	52	54		
	1470				121	54	57		
	1200				150	56	94		

Ultimate load 1470 lb.

Broke into two pieces, some longitudinal splitting. Angles 31° and 30°.

<u>3C10,000</u>									
1	2	3	4	5	6	7	8	9	10
0	0	120			4				
2500	0	238			15				
5000	0	329			25				
7500	0	421			34				
10000	0	503			44				
	200	503			49	46	3		
	400				57	48	9		
	600				65	50	15		
	800				72	52	20		
	1000				79	54	25		
	1200				87	56	31		
	1400				97	58	39		
	1420				115	60	55		
	1000				150	62	88		

Ultimate load 1420 lb.

Piece damaged.

3C12, 500

1	2	3	4	5	6	7	8	9	10
0	0	110			6				
2500	0	237			14				
5000	0	330			23				
7500	0	416			33				
10000	0	496			44				
12500	0	579			60				
	200	579			63	62	1		
	400		3	3	75	64	11	8	3
	600	578	3	6	88	66	22	16	6
	800		3	9	100	68	32	23	9
	1000	577	3	12	113	70	43	31	11
	1200	575	18	30	126	72	54	24	9
	1400	573	13	43	143	74	69	26	10
	1600	572	13	56	165	76	89	33	12

Broke into three pieces, some longitudinal splitting. Angles 30° and 24°.

3C15, 000

15000	0	406			40				
	200	405.5			49	42	7		
	400	405			58	44	14		
	600	404			67	46	21		
	800	403			77	48	29		
	1000	402			87	50	37		
	1200	401			99	52	47		
	1400	400			111	54	57		
	1600	399			124	56	68		

Ultimate load 1600 lb.

Broke into two pieces. Angles 30° and 28°.

<u>3C17, 500</u>									
1	2	3	4	5	6	7	8	9	10
2500	0								
5000	0	129			29				
7500	0	210			38				
10000	0	290			47				
12500	0								
15000	0	427			65				
17500	0	486			79				
	200				84	80	4		
	400				92	82	10		
	600	486			101	84	17		
	800	485.5			110	86	24		
	1000	485			120	88	32		
	1200	484			130	90	40		
	1400	483			140	92	48		
	1600	482			155	94	61		
	1800	481			170	96	74		
	1900	479			190	98	92		
	1910	477			220	100	110		

Ultimate load 1910 lb.

Broke into two pieces. Angles 30°

		<u>IE0</u>									
1	2	3	4	5	6	7	8	9	10		
0	200				18	8	10				
0	400				25	10	15				
0	600				31	12	19				
0	800				41	14	27				
0	950				54	16	38				

Ultimate load 950 lb

Two pieces with some small pieces angle 30°

		<u>IE 2500</u>									
2500	Piece damaged	3	4	5	6	7	8	9	10		
0	0	321	8	8	15	17	7	1	0		
200	200	320	3	11	24	19	14	3	1		
400	400	319.5	3	14	33	21	20	6	2		
600	600	318.7	3	17	41	23	28	11	4		
800	800	318	3	25	50	25	35	10	4		
1000	1000	317	8	33	60	27	43	10	4		
1040	1040	316	8		70	31	67	10	4		
1000	1000	312.5			98			3	1		

Ultimate load 1040 lb

Piece damaged

IE 5000

2500	Piece damaged	3	4	5	6	7	8	9	10		
0	0	515			14						
0	0	524.5			22						
200	200	524			32	24	8				
400	400	523.5			40	26	16				
600	600	522.5			49	28	21				
800	800	521.9			57	30	27				
1000	1000	521			66	32	34				
1200	1200	520			77	34	43				
1400	1400	519			87	36	51				
1600	1600	516.5			100	38	62				
1800	1800				130	40	90				

Ultimate Strength 1800 lb

Broke into two pieces angle 29°

<u>IE 10000</u>									
1	2	3	4	5	6	7	8	9	10
2500	0	140			6				
5000	0	284			13				
7500	0	375			19				
10000	0	458			25				
10000	200	546			34				
10000	400	545	3	3	43	36	7	4	1
10000	600	544	3	6	55	38	17-	11	1
10000	800	543	3	9	67	40	27	18	7
10000	1000	542	3	12	79	42	37-	25	9
10000	1200	540	8	20	92	44	48	28	10
10000	1400	538.5	13	33	106	46	60	28	10
10000		536.5	18	51	126	48	78	27	10

Ultimate load 1580 lb

Broke two pieces; major piece angles 22° the other 34°

<u>IE 75000</u>									
2500	0	200			10				
5000	0	328			20				
7500	0	414			28				
7500	200	504.5			37				
7500	400	503	3	3	45	30	7-	4	1
7500	600	502.5	8	11	53	32	13	2	1
7500	800	501.5	8	19	62	34	19	0	0
7500	1000	502.5-503.5	8	27	71	36	26-	1	0
7500	1200	502.5	8	35	81	38	33	2	1
7500	1400	501.5	8	43	91	40	41	2	1
7500	1600	500.5	8	51	106	42	49	2	1
7500	1700	499.5	8	59	126	44	62	3	1
7500		477.5	18	77		46	80	3	1

Ultimate load 1700 lb

Broke into two pieces Angle 32°

IE 17500

1	2	3	4	5	6	7	8	9	10
2500	0	116			19				
5000	0	214			22				
7500	0	292			26				
10000	0	375			35				
12500	0	450			42				
15000	0	510			49				
17500	0	573.5			58				
17500	200	573			67	60	7		
17500	400	572			77	62	15		
17500	600	571-576.5			88	64	24		
17500	800				95-100	66	29		
17500	1000	574.5			109	68	41		
17500	1200	573.5			119	70	49		
17500	1400	572.5			129	72	57		
17500	1600	571.5			141	74	67		

Ultimate load 1600 lb

Broke into two pieces Angles 34° and 33°

2E0

0	200	12	8	4
0	400	16.5	10	6.5
0	600	21.5	12	9.5
0	800	25.5	14	11.5
0	1000	30.5	16	14.5
0	1200	35	18	17
0	1300	41	20	21

Ultimate load 1300 lb

Broke into two pieces Angle 35°

2E 2500

1	2	3	4	5	6	7	8	9	10
2500	0	305	8	8	20	22	12	4	1
2500	200	304	8	16	34	24	20	4	1
2500	400	303	8	29	44	26	27	3	1
2500	600	302	8	32	53	28	33-	1	1
2500	800	301	8	40	61	30	40-	0	0
2500	1000	300	8	53	71	32	48	0	0
2500	1200	298.5	13	66	81	34	59	0	0
2500	1400	297	13	77	93	36	77	1	0
2500	1600	295.7	11		113				

Ultimate load 1600 lb

Broke into two pieces angles 34° and 30°

2E 7500

2500	0	257	8	8	14	33	13	13	5
5000	0	338	18	26	23	35	30-	4	1
7500	0	423.5	13	39	31	37	51	12	4
7500	200	422.5	8	47	46	39	63	16	6
7500	400	420.5	13	60	65	41	73-	3	1
7500	600	418	13	73	88	43	83	10	4
7500	800	417	13	86	102	45	99	13	5
7500	1000	415.5	13		114				
7500	1200	414	13		126				
7500	1400	412.5	13		144				

Ultimate load 1400 lb

Broke into two pieces angle 30°

2E 5000

2500	0	290	8	8	14	24	10-	2	1
5000	0	384.5	13	21	22	26	19	2	1
5000	200	383.5	8	29	34	28	29	0	0
5000	400	382	8	37	44	30	38	1	0
5000	600	381	8	50	57	32	48	2	1
5000	800	380	13	58	68	34	57	1	0
5000	1000	378.5	8	81	80	36	87	6	2
5000	1200	377.5	23		91				
5000	1400	375			123				

Ultimate load 1400 lb

Broke into two pieces angles 29° and 23°

	<u>2E 10000</u>									
	1	2	3	4	5	6	7	8	9	10
2500		0	149			9				
5000		0	238			14				
7500		0	312			24				
10000		0	400			36				
10000		200	400			45	37			
		400	399			55	39	8		
10000		600	378			66	41	16		
		800	397			75	43	25		
10000		1000	396			85	45	32		
10000		1200	395			96	47	40		
		1400	394			108	49	49		

Ultimate load 1600 lb

Broke into three major pieces Angle 24° and 16°

	<u>2E 12500</u>									
2500	0	234								13
5000	0	325								21
7500	0	401								31
10000	0	480								41
12500	0	561								50
	200	560								60
12500	400	559								70
	600	559								76
12500	800	558.5								84
	1000	557.5								73
12500	1200	557								104
12500	1400	556								116
12500	1600	554.5								130

Ultimate load 1600 lb

Broke into two pieces angle = 33°

2E 17500

1	2	3	4	5	6	7	8	9	10
2500	0	80			10				
5000	0	173			15				
7500	0	263			23				
10000	0	338			30				
12500	0	4413			38				
15000	0	5480			44				
17500	0	5548			47				
	200	547.5	3	3	54	49	5	5	2
	400	547	3	6	62	51	12	6	2
17500	600	546.5	3	9	70	53	17	8	3
	800	546	3	17	78	55	23	6	2
	1000	545	8	25	86	57	29	4	1
	1200	544	8	33	97	59	38	5	2
	1400	543	8	41	109	61	48	7	3
17500	1600	542	8	49	123	63	60	12	4
	1800	540	18	67	144	65	79	22	8

Broke into two pieces angle 34° Ultimate load 1840 lb

2E 15000

2500	0	151			18				
5000	0	239			23				
7500	0	325			28				
10000	0	409			335				
12500	0	485			389				
15000	0	559			48				
					59	50	9-	9	3
15000	400	558	8	8	69	52	17	9	3
	600	557	8	16	78	54	24	8	3
	800	556.5	3	19	84	56	28	9	3
	1000	556.0	3	22	93	58	35	13	5
15000	1200	555	8	36	102	60	42	12	4
	1400	554	8	38	114	62	52	14	5
	1600	553	8	46	126	64	62	16	6
	1800	552	8	54	140	66	74	20	7
	2000				158	68	90		

Ultimate load 2000 lb

Piece Damaged

3EO

1	2	3	4	5	6	7	8	9	10
0	200				15	8	7		
0	400				19.5	10	9.5		
0	600				24	12	12		
0	800				28	14	14		
0	1000				33	16	17		
0	1100				40	18	22		
					45	20	25		

Ultimate load 1160 lb

Broke into two pieces 33°

3E 2500

2500	0	207	8	8	24	26	10	2	1
2500	200	205	8	16	36	28	17	1	0
	400	204	13	29	45	30	27	2	1
2500	600	203.5	8	37	57	32	35	2	1
	800	202.5	8	45	67	34	43	2	1
	1000	201.5	13	58	77	36	52	6	2
2500	1200	200.5	8	66	88	38	62	4	1
	1400	199.5	13	79	100	40	80	1	0
2500	1600	198	13		120				

Piece Damaged

Ultimate load 1600 lb

3E 5000

2500		152			20		8		2
5000		237	3	3	32	34	16	5	2
	200	236.5	8	11	42	36	23	5	2
5000	400	235.5	8	19	52	38	30	4	1
	600	234.5	8	27	61	40	37	3	1
5000	800	233.5	8	35	70	42	44	2	1
	1000	232.5	8	43	79	44	53	1	0
5000	1200	231.5	8	51	88	46	62	2	1
	1220	230.5	8	59	99	48		3	
5000	1200	229.5	8		110				177

Ultimate load 1220 lb

Broke into three pieces angles = 34° and 22°

	1	2	3	4	5	6	7	8	9	10
					<u>3E7500</u>					
2500		0	393			22				
5000		0	490			36				
7500		0	573.5			50				
		200				58	52	6		
7500		400				66	54	12		
		600				74	56	18		
7500		800				82	58	24		
		1000				90	60	30		
		1200				100	62	38		
		1400				112	64	48		
7500		1460				131	66	65		

Ultimate load 1460 lb

[illegible]

Ultimate load 1800 lb

Piece Damaged

3E 15000

1	2	3	4	5	6	7	8	9	10
2500	0	175			14				
5000	0	255			24				
10000	0	412			45				
15000	0	553			66				
	200	552.5-561	3	3	76	68	08-	5	2
15000		55							
	400	560	8	11	85	70	15	4	1
15000	600	559	8	19	95	72	23-	4	1
	800	558.5	3	22	103	74	29	6	2
	1000	558	3	25	113	76	37	12	4
15000	1200	557	8	33	123	78	45	12	4
	1400	556	8	41	135	80	55	14	5
15000	1600	554.5	13	54	152	82	70	16	6

Ultimate load 1600 lb

Broke into two pieces 32°

3E 17500

1	2	3	4	5	6	7	8	9	10
2500		125			24				
5000		215			29				
10000		373			47				
15000		415			65				
17500		577.5			74				
	200	577	3	3	86	76	10	7	3
17500	400	576	3	6	92	78	14-	6	2
	600	575.5	8	14	99	80	19	5	2
	800	574	13	27	107	82	25	0	
	1000	573	8	35	117	84	33-	0	
17500	1200	572.5	3	38	127	86	41	3	1
	1400	571.8	6	44	139	88	51	7	3
17500	1600	570.5	11	55	152	90	63	8	3

Ultimate load 1810 lb

Broke into two pieces 34°

	<u>4E 15000</u>									
	1	2	3	4	5	6	7	8	9	10
2500		0	183			20				
5000		0	268			29				
7500		0	42							
10000		0	424			48				
12500		0	497			58				
15000		0	567			68				
		200	567.5	3	3	76	70	6	3	1
15000		400	566	3	6	86	72	14	8	3
		600	565.5	3	9	96	74	22	13	5
15000		800	565	3	12	105	76	29	17	6
		1000	564	3	15	115	78	37	22	8
		1200	563	8	23	125	80	45	22	8
15000		1400	562	8	31	136	82	54	23	9
		1600	561	8	39	150	84	66	27	10
		1700	559.5	13	52	165	26	79	27	10
15000		1000	556	33	65	200	88	112	47	17
		800	550	60	125	260	90	170	45	17

Ultimate load 1700 lb

	<u>4E 17500</u>									
	1	2	3	4	5	6	7	8	9	10
2500		0	77			20				
5000		0	162			30				
7500		0	250			38				
10000		0	330			47				
12500		0								
15000		0	462			64				
17500		0	533.5			74				
		200	533	3	3	82	76	6	3	1
17500		400	533	8	11	97	78	13	2	1
		600	533	0	11	99	80	19	8	3
17500		800	533	0	11	106	82	25	7	3
		1000	532.5	0	11	115	84	31	20	7
17500		1200	532	3	14	125	86	39	25	9
		1400	531							
		1600	530.5	22	36	145	90	55	19	7
17500		1750	529.5	8	44	155	92	62	18	7
		1400	528	13	57	175	94	91	24	9

Piece damaged Ultimate load 1700 lb

		<u>1G0</u>									
1	2	3	4	5	6	7	8	9	10		
0	0										
200					10	8	2				
400					16	10	6				
600					21	12	9				
800					26	14	12				
920					33	16	17				

Ultimate load 920 lb.

Broke into two pieces. Angle 29°

		<u>1G2500</u>									
2500		3	4	5	6	7	8	9	10		
0	0										
200	167		8		20	22	12	4	1		
400	166		8	16	34	24	21	5	2		
600	165		13	29	45	26	31	2	1		
800	163.5		13	42	57	28	57	15	6		
900	162		29	71	75	30	75	4	1		
600	159		24	95	105		102	7	3		
	156				135						

Ultimate load 920 lb.

Piece damaged.

1G5000

1	2	3	4	5	6	7	8	9	10
2500	0	170			20				
5000	0	255			31				
	200					33			
	400	253	16	16	60		25	9	3
	600	252	8	24	71	35	33	9	3
	800	251	8	32	85	39	46	14	5
	900	249	18	50	105	41	64	14	5

Ultimate load 900 lb.

Broke into two pieces, small pieces. Angle 28°.

1G7500

2500	0	59			20				
5000	0	156			33				
7500	0	230.5			45				
	200	237.5	8	8	59	47	12	4	1
	400	236.5	8	16	73	49	24	8	3
	600	235	13	29	85	51	34	5	2
	800	233.5	13	42	97	53	44	+2	1
	1000	231	23	65	118	55	63	-2	1
	1200	227	29	94	155	56	99	5	2
	600	222	50	144	200		146	2	1

Ultimate load 1200 lb.

Broke into three pieces. Angles 31° and 22°

	<u>1G10, 000</u>									
	1	2	3	4	5	6	7	8	9	10
2500		0	185			20				
5000		0	274			31				
7500		0	351			40				
10000			433.5			50				
		200	432.5	8	8	62	52	10	2	1
		400	431.5	8	16	74	54	20	4	1
		600	430	13	29	85	56	29	0	
		800	429	8	37	99	58	41	4	1
		1000	427	18	54	120	60	60	6	2

Ultimate load 1150 lb.

Broke into two pieces. Angle 28°

	<u>1G12, 500</u>									
	1	2	3	4	5	6	7	8	9	10
2500		0	160			20				
5000		0	252			30				
7500		0	337			40				
10000		0	415			51				
12500			490			57				
		100	489	8	8					
		200	488.5	3	11	67	59	8	3	1
		300	488	3	14					
		400	487.5	3	17	79	61	18	3	1
		500	487.0	4	21					
		600	486	7	28	91	63	28	0	0
		800	485	8	36	103	65	38	2	1
		1000	483.5	13	49	120	67	53	4	1
		1090	480	34	83	155	68	87	4	1
		300	470	97	180	253		185	5	2

Ultimate load 1090 lb.

Piece damaged.

1G15,000

1	2	3	4	5	6	7	8	9	10
2500	0				40				
5000	0				52				
7500	0								
10000	0								
12500	0								
15000	0				73				
	200	253			79	75	4		
	400	254.5			94	77	17		
	600	253.5			109	79	30		
	800	252			125	81	44		
	1000	250			145	83	62		

Ultimate load 1100 lb.

Broke into two pieces. Angle 30°.

1G17, 500

1	2	3	4	5	6	7	8	9	10
2500	0	70			20				
5000		163			33				
7500	0	260			47				
10000		321			57				
12500	0								
15000		456			74				
17500	0	524			81				
	200	524			89	83	6	6	2
	400	523			96	85	11	11	4
	600	523.5	3	3	105	87	18	15	6
	800	523	3	6	113	89	24	15	6
	1000	522.5	3	9	124	91	33	16	6
	1200	521.5	8	17	140	93	47	17	6
	1400	520	13	30	150	95	55	7	3
	1450	518	18	48	178	97	81		
		---stays slowly decreases							
		504	90	138	267	92	175	37	12

Ultimate load 1450 lb.

Broke into two pieces. Angle 28°

2G0

1	2	3	4	5	6	7	8	9	10
0	200				12	8	4		
0	400				19	10	9		

Ultimate load 400 lb

Broke two pieces Angle 27°

2LG 2500

2500	0	455			20	22	11		
	200	455			33	24	18		
2500	400	454			42	26	27		
	600	453			53	28	38		
	800	451.5			66	30	56		
2500	1000	458.5			86		80		
	1120	458			110		110		
2500	600	435			143				

Ultimate load 1120 lb

Broke into two pieces; some small pieces Angle 28°

2G5000

2500	0	209			20	37	9-		
5000	0	308			35	39	15		
	200	307.5			46	41	22		
5000	400	306.5			54	43	31-		
	600	305.5			63	45	40		
	800	309.5			74	47	50		
	1000	303.5			85	49	68		
	1200	302.0			97	50	105		
5000	1330	300			117	50	160		
	800	296			155				
5000	600	292			210				

Ultimate load 1330 lb

Broke into two pieces. Angle 27°. Some small pieces.

2G 7500

1	2	3	4	5	6	7	8	9	10
2500	0	289			20				
5000	0	376			36				
7600	0	467			51				
	200	466			65	53			
7600	400	464.5			79	55	12		
	600	463.5			92	57	24		
7600	800	462.5			103	59	35		
	1000	461.5			115	61	44		
	1040	460			131	63	54		
7600	1000	468.5			152	65	68		

Ultimate load 1040 lb

Broke two pieces. Angle 29°

2G 12500

2500	0	73			20				
5000	0	171			29				
7500	0	240			37				
1000	0	329			47				
12500	0	402.5			57				
	200				66	59	7		
12500	400				75	61	14		
12500	600	401.5			84	63	21		
	800	401			94	65	29		
12500	1000	400			107	67	40		
	1200	398			125	69	56		
	1300	395			160	70	90		
12500	1000	391			200		130		
	600	388			255		185		

Ultimate load 1300 lb

	<u>2G 10000</u>									
	1	2	3	4	5	6	7	8	9	10
2500		0	228			20				
5000		0	325			31				
7500		0								
10000		0	489			49				
10000		200	489			60	51	9		
		400	488.5			68	53	15		
		600	488			78	55	23		
		800	487.5			87	57	30		
10000		1000	486.5			95	59	36		
		1200	485.5			110	61	49		
10000		1400	484			125	63	62		
		1000	482			155	65	90		

Broke into two pieces. Angle 30° Ultimate load 1400 lb

	<u>2G 15000</u>									
2500		0	188			20				
5000		0	276			28				
7500		0	357			36				
10000		0	425			47				
12500		0	488			54				
15000		0	489			67				
		200	489			77	69	8		
		400	488.5			88	71	17		
		600	488			98	73	25		
		800	487.5			107	75	32		
		1000	486.5			117	77	40		
		1200	485.5			131	78	52		
		1400	484			145	81	64		
		1000	482			168	83	85		
		1700	480			190	84	<u>106</u>		
		<u>1200</u>	475			260		165		
		800	472			390		305		

<u>2G1 7500</u>									
1	2	3	4	5	6	7	8	9	10
2500	0	32			20				
5000	0	136			36				
7500	0	227			45				
10000	0	300			55				
12500	0	373			63				
15000	0	439			71				
17500	0	505			80				
	200	505			88	82	6		
17500	400	505			95	84	11		
	600	504			105	86	19		
	800	503			116	88	28		
17500	1000	502			130	90	40		
	1100	500			151	92	59		
	1160	497			187	93	94		
17500	800	492			242				

Ultimate load 1160 lb

3G0

0	200				17	8	9		
0	400				24	10	14		
0	570				31	12	19		

Ultimate load 570 lb
Broke into two pieces. Angle 29°

3G 2500

2500	0	158			13				
	200	149			25	15	10		
	400	148			36	17	19		
	600	147			48	19	29		
2500	800	146			65	21	44		
	1000	144			88	23	65		

Ultimate load 1000 lb
Broke into two pieces. Some small pieces Angles 30° and 29°

	<u>3G 5000</u>									
	1	2	3	4	5	6	7	8	9	10
2500		0	220			21				
5000		0	300			32				
		200	299			39	34	05		
5000		400	298			51	36	14		
		600	297			63	38	25		
5000		800	296			81	40	41		
		1000	295			104	42	62		
5000		1100	293			120	44	76		

Broke into two pieces Angle 22° Some small pieces.

	<u>3G 10000</u>									
	1	2	3	4	5	6	7	8	9	10
2500		0	170			7				
5000		0	269			12				
7500		0	351			15				
10000		0	432			21				
		200	432			33	23	10		
10000		400	431			45	25	20		
		600	430			58	27	31		
10000		800	429			71	29	42		
		1000	428			85	31	54		
10000		1200	426			99	33	66		
		1300	424			114	35	79		

Ultimate load 1300 lb

Broke into two pieces Angle 29°

	<u>3G 12500</u>									
	1	2	3	4	5	6	7	8	9	10
	2500	0	53			20				
	5000	0	149			34				
	7500	0	227			44				
	10000	0	299			54				
	12500	0	373			63				
		200	373			70	65	5		
	12500	400	372.5			81	67	14		
		600	372			91	69	23		
		800	371.5			102	71	31		
		1000	370.5			112	73	39		
		1200	369.5			125	75	50		
		1400	368			142	77	65		
	12500	1600	363			198	79	119		
		1200	359			240		160		
		1000	357			268		190		

Ultimate load 1600 lb

Piece damaged

	<u>3G 15000</u>									
	1	2	3	4	5	6	7	8	9	10
	2500	0	170			20				
	5000	0	250			30				
	7500	0								
	10000	0	407			47				
	12500	0								
	15000	0	548			68				
	15000	200	548			78	70	8		
		400	447.5			91	72	19		
	15000	600	546.5			104	74	30		
		800	545.5			118	76	42		
		1000	544			133	78	55		
	15000	1200	542.5			150	80	70		
		1350	540			164	81	83		

Ultimate load 1350 lb

Price damaged -

3G 17500									
1	2	3	4	5	6	7	8	9	10
2500	0	130			20				
5000		220			26				
7500	0								
10000	0	378			43				
12500									
15000	0	420			62				
17500	0	582			72				
17500	200	581			81	74	7		
	400	580			98	76	22		
17500	600	578			115	78	37		
	800	576			134	80	54		
	1000	574			156	82	74		
17500	1200	572			179	84	95		
	1300	570			196	86	110		

Broke into two pieces. Angle 28° Ultimate load 1300 lb

110

0	200	22	8	14
0	400	33	10	23

Ultimate load 400 lb

Broke into many pieces.

112500

2500	0	69	13	13	20	22	18-	6	2
	200	67.5			40				
2500	400	66	8	21	55	24	31	10	4
	600	65	8	29	69	26	43	14	5
2500	800	63.5	13	42	82	28	54-	12	4
	1000	62	13	55	100	30	70	15	6
	1200	59	28	83	130	32	98	15	6

Broke into three pieces. Angles 24° and 18° Ultimate load 1240 lb

11 5000

1	2	3	4	5	6	7	8	9	10
2500	0	226							
5000	0	332			20	22	11-		
5000	200	331			33	24	30		
5000	400	330			54	26	36		
5000	600	299.5			62	28	46-		
5000	800	299			74	30	58		
5000	1000	298			88	32	71		
	1200	297			103				

Ultimate load 1200 lb

Broke into two major pieces with several small pieces angle 22°

11 7500

2500	0	322			20	59	9-	6	2
5000	0	411			34	61	17		
7500	0	515			57	63	27	21	8
7500	200	514.5	3	3	68	65	40-	26	10
	400			6	78	67	53	31	11
7500	600	514	3	14	90	69	69	34	13
	800	513	8	22	105	71	94	41	15
	1820	512	8	35	120				
7500	1200	510.5	13	53	138				
	1400	508.5	18		165				

Ultimate load 1440 lb

Broke into two major pieces Angle 22°

11 10000

2500	0	60			20	81	18-	10	4
5000	0	147.5			35	83	38	17	6
7500	0	146.5			57	85	54	25	9
10000	0	145	8	8	79	87	72-	35	13
	200	144	13	21	99	89	88	38	14
10000	400	143	8	29	121	91	104	41	15
	600	141.5	13	37	139				
10000	800	140	13	50	159				
	1000		13	63	177				
10000	1050		13		195				

Ultimate load 1050 lb

Piece damaged

11 12500										
1	2	3	4	5	6	7	8	9	10	
2500	0	184			20					
5000	0	276			36					
7500		361			57					
10000	0	437			80					
12500	0	523			129					
	200	523			144	131	13-			
12500	400	522.5	3	3	162	133	29-	26	10	
	600	521.5	8	11	181	135	46	35	13	
	800	520.5	8	19	204	137	67	48	17	
12500	1000	519	13	32	223	139	84	52	19	
	1200	517	18	50	250	141	109	59	22	
	1220	515.5	13	63	270	143	127	64	24	

Ultimate load 1220 lb

11 15000

11 15000										
2500	0	12			21					
5000	0	102			31					
10000	0	259			62					
12500	0	343			89					
15000	0	415			140					
	200				150	142	8-			
	400	414.5	3	3	166	144	22-	19		
15000	600	414	3	6	181	146	35	29		
	800	413	8	14	202	148	54	40		
15000	1000	412	8	22	222	150	72	50		
		410.5	13	35	245	152	93	58		
	1280	409	13	48	270	154	116	68		
15000	1000	407.5	13	61	291	156	135	74		

Ultimate load 1280 lb

Broke into two pieces. Angle 24°

11 17500

1	2	3	4	5	6	7	8	9	10
2500	0	54			20				
5000		148			28				
7500	0	244			40				
10000	0								
12500		286			80				
15000	0	467			100				
17500		532.5			125				
	200	531.5	8	8	151	127	24		
17500	400	530	13	21	171	129	42		
	600	529	8	29	188	131	57		
	800	528	8	37	206	133	73		
17500	1000	526.5	11	48	222	135	87		
						137			
	1200	523.5	24	72	238	139	99		
17500	1300	521.5	16	88	275	41	134		

Ultimate load 1300 lb

Piece damaged.

210

0	200				16	8	8		
0	400				26	10	16		
0	500				37	12	25		

Ultimate load 500 lb

Broke into two major pieces with some small pieces Angles 23° and 21°

2500	0	240.5			20				
	200	239.5			36	22	14-		
2500	400	238			49	24	25		
	600	237			61	26	35		
	620	235.5			75	28	47-		
2500	400	233.5			92	30	62		

Ultimate load 620 lb

Broke into two pieces with some small pieces angle 21°

	1	2	3	4	5	6	7	8	9	10
					<u>21 5000</u>					
	2500	0	267			20				
	5000	0	362			29				
	5000	200								
	5000	400	360			52	33	19		
	5000	600	359			62	35	27		
	5000	200	358			75	37	38		
	5000	900	356			93	39	54		

Ultimate load 900 lb

					<u>21 7500</u>					
Piece damaged.										
	2500	0	160			20				
	5000	0	240			30				
	7500	0	329			38				
		200	328			51	40	11-		
	7500	400	327.5-339			65-69	46	23		
		600	337.5			82	48	34		
	7500	800	336.5			96	50	46-		
		1000	335			112	52	60		
		1200	333			133	54	79		
	7500	1260	330			170	56	114		

Ultimate load 1260 lb

Broke into two pieces Angle 20°

					<u>21 10000</u>					
	2500	0	240			20				
	5000	0	326			30				
	7500	0	415			42				
	10000	0	492			54				
		200	491.5			66	56	10		
	10000	400	491			73	58	15		
		600	490.5			85	60	25-		
		800	490			98	62	36		
	10000	1000	490.5			115	64	51		
		1200	498			135	66	69		
	10000	1250	496			170	68	102		
		1000	494			190	70	120		

Ultimate load 1250
Piece damaged

21 12500

1	2	3	4	5	6	7	8	9	10
2500	0	198			20				
5000	0	283			32				
7500	0	373			42				
1000	0	444			54				
12500	0	520			64				
	200	520			73	66	7-		
12500	400	519			82	68	14-		
	600				93	70	23-		
12500	800	518			105	72	33-		
	1000	517			120	74	46		
	1200	514.5			195	76	69		
12500	1360	510			195	78	<u>117</u>		
	1000	508			230	80	15		

Ultimate load 1350 +

21 17500

1	2	3	4	5	6	7	8	9	10
2500	0	51			20				
5000	0	142			25				
7500	0	220			35				
10000	0	295			46				
12500	0	374			58				
15000	0	441			68				
17500	0	500			77				
	200				81	79	3-		
	400				91	81	10		
17500	600	499.5			100	63	17		
	800	499			111	65	26-		
17500	1000	498.5			125	86	38		
	1200	497.5			143	89	54		
17500	1400	495			168	91	77		
17500	1500	491.5			210	92	118		
	1000	485			280	94	186		

Ultimate load 1500 lb.

Broke into two pieces, Angle 28°

<u>21 15000</u>									
1	2	3	4	5	6	7	8	9	10
2500	0	164			20				
5000	0	255			26				
7500		332			40				
1000		413			52				
12500	0	486			63				
1500		557			75				
	260	557			86	77	9-		
	400	557			92	79	13		
	600	556.5			104	81	23-		
	800	556			116	83	33-		
	1000	555.5			131	85	45		
	1200	554			155	87	67		
	1400	548			210	89	121		
	1000	540			285	91	194		

Ultimate load 1400 lb

Broke into two pieces angle 21°

310

1	2	3	4	5	6	7	8	9	10
0	200				14	8	6		
0	300				16	10	16		

Broke into two pieces. Angle 24° Ultimate load 300 lb

31 2.5

2500	0	175			20	22	19		
	200	173			41	24	31		
2500	400	171.5			55	26	41		
	600	170			67	28	52		
2500	800	168.5			80	30	67		
	1000	167			97	32	108		
2500	1140	163			140				

Ultimate load 1140 lb

31 5000

2500	0	109			20	35	9		
5000	0	208			33	37	25		
5000	200	207			44	39	42		
	400	206			62	41	59		
5000	600	204.5			81	43	76		
	800	203			100				
5000	900	201			119				

Ultimate load 920 lb

Broke into two pieces, with some small pieces. Angles 23° and 22°.

31 7500

2500	0	78			20	38	9		
5000	0	166			28	40	19		
75000	0	260			36	42	28		
	200	260			47	44	39		
75000	400	259.5			59	46	54		
	600	258.5			70	48	112		
75000	800	257			83	50	150		
	1000	255.5			100				
75000	1200	254			160				
	800	244			200				

Broke into two pieces with some small pieces. Angle 24° Ultimate load 1200 lb

31 10000

1	2	3	4	5	6	7	8	9	10
2500	0	180			20				
5000	0	265			28				
7500	0								
10000	0	419			48		4		
	200	418.5			54	50	21		
10000	400	418			73	52	38		
	600	416.7			92	54	54		
	800	415			110	56	72		
10000	1000	413			130	58	85		
10000	1200	411			145	60			

Broke into three major pieces. Angle 20°. Ultimate load 1270 lb

31 12500

2500	0	70			20				
5000	0	159			30				
7500	0	250			38				
10000	0	330			46				
12500	0	421			52				
	200	420			71	54	17		
12500	400	418.5			91	56	35		
	600	417			109	58	51		
	800	415			128	60	68		
12500	1000	413.5			147	62	85		
	1200	413			168	64	104		
	1280	410			178	66	112		

Ultimate load 1280 lb

Broke into two pieces. Angle 31°

31 17500

1	2	3	4	5	6	7	8	9	10
2500	0	63			20				
5000	0	156			27				
7500	0	243			37				
10000									
12500	0	382			58				
15000	0	452			66				
17500	0	519			75				
	200				82	77	5-		
17500	400	518.5			90	79	11		
	600	518			100	81	19		
17500	800	517			110	83	27-		
	1000	516			123	85	38		
	1200	515			110	87	53		
	1400	512			170	89	81		
17500	1480	510			195	90	105		
	1000	505			255	92			

Ultimate load 1480 lb

Piece damaged

Appendix 3

FAILURE OF A MATERIAL COMPOSED OF NON-ISOTROPIC ELEMENTS

by Anton Brandtzaeg

Mr. Brandtzaeg's failure theory has been discussed in the analysis section. In this section an abstract of the theory will be given. Mr. Brandtzaeg considers an elastic material with a constant Poisson's ratio, ν , and modulus of elasticity, E . All elements have different directions of weaknesses and until a limiting tensile stress occurs they can only fail with a plastic sliding at a limiting shearing stress and that action is governed by Coulomb's equation.

$$\tau_{lim} = \tau_0 \tan \theta(\sigma) \quad (18)$$

In this theory different notations for elements and material have to be used to differentiate between the two. Notations for the material are the same as before, and for elements will be presented as they occur. In Equation 18 σ is the normal stress, τ_0 is the shearing strength, θ is the angle of internal friction, and τ_{lim} is the limiting shearing stress in the elements.

From figure 35a the stresses on a plane in the direction of weakness are:

$$\sigma = \sigma_1 \sin^2 \phi + \sigma_2 \cos^2 \phi \quad (19)$$

$$\tau = \frac{\sigma_1 - \sigma_2}{2} \sin 2\phi \quad (20)$$

$$\sigma_v = \frac{1}{3}(\sigma_1 + 2\sigma_2) \quad (21)$$

where σ_1 and σ_2 are the normal stresses in axial and lateral directions respectively. Combining equations 19 and 21:

$$\sigma = \sigma_v + \frac{\sigma_1 - \sigma_2}{2} \left(\frac{1}{3} - \cos 2\phi \right) \quad (22)$$

where $\tan \theta = \cot 2\theta$ and $\phi_0 = \frac{\pi}{4} - \frac{\theta}{2}$ introducing angle and

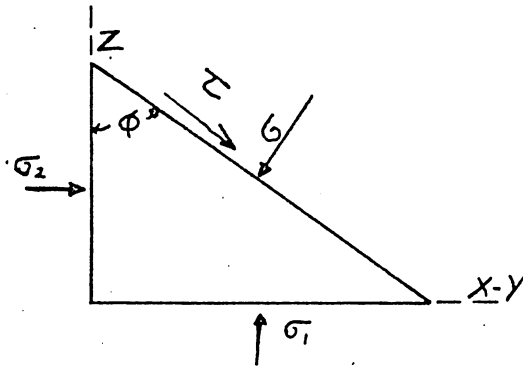
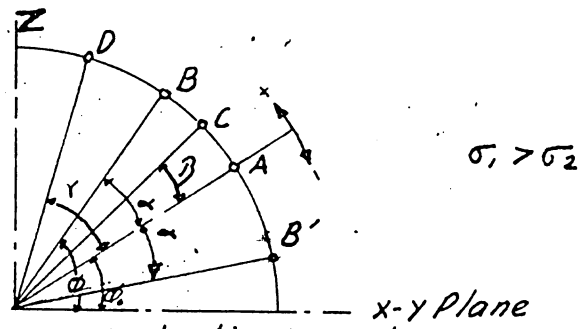


Figure 35 (a)



$B \text{ to } B, \text{ plastic elements}$
 $x-y \text{ Plane to } B, \text{ elastic elements}$
 $B \text{ to } z \text{ axis}$

Figure 35 (b)

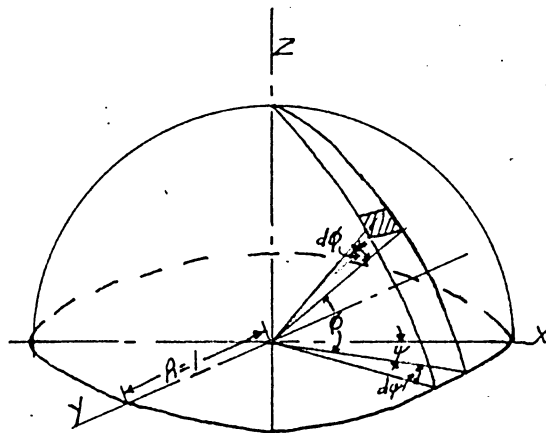


Figure 35 (c)

combining equations 18 and 20 and 22:

$$\left(\frac{\sigma_1 - \sigma_2}{2} \right)_{lim} = \frac{K + F\sigma_v}{\cos 2(\phi - \phi_0) - \frac{1}{3}F} \quad (23)$$

where $K = \tau_0 \sin 2\phi_0$; $F = \cos 2\phi_0$

This is the necessary condition for plastic equilibrium and a limiting value that the difference between the principal stresses can never exceed. If it is below this limit the element is elastic and as long as the stress difference is equal to this limit the element deforms plastically. Elements that have the lowest limiting stress difference values will reach the plastic stage first, for which ϕ will be equal to ϕ for $\sigma_1 > \sigma_2$.

Now consider figure 35b where plasticity spreads from point A to both directions. At A, $\phi = \phi_0$ where the elements first started to plastic action are represented. Points between B and B', such as C, defined by angle β , have been deforming plastically for some time. Elements B and B' with an angle α are just becoming plastic. Elements between the x-y plane and B' or between the z-axis and B are elastic.

α : is a constant which defines the extent that loading has made the material plastic.

β : is a variable for a given stress situation defining plastic elements with the same direction of weaknesses.

γ : is a variable for a given stress situation defining elements in the elastic range.

Suffix γ and β are used for stresses in elastic and plastic elements respectively. Substituting in equation 23 we get stress difference equations for elastic and plastic elements. Stresses in elastic elements are the same as in an element just passing the plastic stage. Combining

equations 21 and 23 with $B = \phi - \phi_0$ or $\alpha = \phi - \phi_0$ for plastic and elastic elements respectively the principal stresses in an elastic element are:

$$\sigma_{\delta_1} = \sigma_v + \frac{2}{3}(\sigma_{\delta_1} - \sigma_{\delta_2}) = \sigma_v + 4 \frac{K + F\sigma_v}{3\cos 2\alpha - F} \quad (24a)$$

$$\sigma_{\delta_2} = \sigma_v - \frac{1}{3}(\sigma_{\delta_1} - \sigma_{\delta_2}) = \sigma_v - 2 \frac{K + F\sigma_v}{3\cos 2\alpha - F} \quad (24b)$$

in a plastic element are:

$$\sigma_{\beta_1} = \sigma_v + 4 \frac{K + F\sigma_v}{3\cos 2\beta - F} \quad (25a)$$

$$\sigma_{\beta_2} = \sigma_v - 2 \frac{K + F\sigma_v}{3\cos 2\beta - F} \quad (25b)$$

Mr. Brandtzaeg assumes elements in a cubical shape with a side length d , so that the number of elements in a unit volume is:

$$N = 1/d^3$$

A unit volume contains all geometrically possible directions of weaknesses of the elements. Mr. Brandtzaeg further assumes normals of unit length from the origin of coordinates, to the direction of the weaknesses of the elements. These elements could be represented as a point on a hemisphere (Fig. 35c), which contains all elements in a unit volume. The number of elements in a small area of the hemisphere:

$$dN = \frac{N}{2\pi} dA = \frac{1}{2\pi d^3} \cos \phi d\phi d\psi \quad (26)$$

In a unit volume N elements exert forces on each other. Some of these balance inside the unit volume. The number of forces that should be balanced by the average stress in the material is $N_f = N$, so that equation 26 becomes:

$$dN_f = \frac{1}{2\pi d^2} \cos \phi d\phi d\psi \quad (27)$$

Substituting into equation 27 $\gamma + \phi_0$ or $\beta + \phi_0$ for ϕ , we find the elementary forces in dN_f elements in an axial direction, exerted by elastic elements:

$$d\tau_{\delta_1} = \frac{1}{2\pi} \sigma_{\delta_1} \cos(\gamma + \phi_0) d\gamma d\psi$$

by plastic elements:

$$df_{\beta_1} = \frac{1}{2\pi} \sigma_{\beta_1} \cos(\beta + \phi_0) d\beta d\psi$$

In a lateral direction, exerted by elastic elements:

$$df_{\gamma_2} = \frac{1}{2\pi} \sigma_{\gamma_2} \cos(\gamma + \phi_0) d\gamma d\psi$$

by plastic elements:

$$df_{\beta_2} = \frac{1}{2\pi} \sigma_{\beta_2} \cos(\beta + \phi_0) d\beta d\psi$$

The principal average stresses may now be obtained by double integration with respect to ψ (between limits 0 and 2π) and γ or β (between $\phi = 0$ and $\phi = \pi/2$). This covers all the hemisphere and as a result all the elements in a unit volume. The equations for the principal average stresses are:

$$f_1 = \sigma_v + 4R\alpha(K + F\sigma_v) \quad (28a)$$

$$f_2 = \sigma_v - 2R\alpha(K + F\sigma_v) \quad (28b)$$

$$f_v = \frac{1}{3}(f_1 + 2f_2) = \sigma_v \quad (28c)$$

where

$$R\alpha = \frac{1 - 2 \cos \phi_0 \sin \alpha}{3 \cos 2\alpha - F} + \frac{\cos \phi_0}{6\sqrt{1/2 - 1/6 F}} \ln \frac{\sqrt{1/2 - 1/6 F} + \sin \alpha}{\sqrt{1/2 - 1/6 F} - \sin \alpha} \quad (28d)$$

Equation 28c shows that volume stress in the material is equal to the volume stress in an element. Combining equations 28a, b, and c:

$$f_1 = f_2 \frac{1 + 4FR\alpha}{1 - 2FR\alpha} + K \frac{6R\alpha}{1 - 2FR\alpha} \quad (29a)$$

$$f_2 = f_1 \frac{1 - 2FR\alpha}{1 + 4FR\alpha} - K \frac{6R\alpha}{1 + 4FR\alpha} \quad (29b)$$

$$f_v = \sigma_v = \frac{f_1 - 4KR\alpha}{1 + 4FR\alpha} = \frac{f_2 + 2KR\alpha}{1 - 2FR\alpha} \quad (29c)$$

By substituting volume stress from equation 29c into equations 24 and 25, stresses in elements could be expressed in terms of principal average stresses. To find deflections, remembering that all elements deform alike, deformation of material is equal to deformation of individual elements. The unit deformation of the material is found by combining

equations 21, 29c and 30 with values of stress differences.

For an elastic element equations of deflection are:

$$\varepsilon_1 = \frac{1}{E}(\sigma_1 - 2\mu\sigma_2) \quad (30a)$$

$$\varepsilon_2 = \frac{1}{E}[\sigma_2 - \mu(\sigma_1 + \sigma_2)] \quad (30b)$$